GENERAL STRUCTURAL NOTES

GENERAL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR AND SHALL VERIFY AND COORDINATE ALL DIMENSIONS AND DETAILS BEFORE PROCEEDING WITH WORK. ANY DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ARCHITECT AND ENGINEERS.
2. DETAILS SHOWN IN ANY SECTION APPLY TO ALL SIMILAR SECTIONS AND CONDITIONS UNLESS NOTED OTHERWISE.
3. CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT ALL WORK IN PROGRESS UNTIL THE BUILDING IS COMPLETED.
4. ALL STRUCTURAL ITEMS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH APPROPRIATE PROVISIONS OF EACH OF THE FOLLOWING:
   A. THE FLORIDA BUILDING CODE. (EIGHTH EDITION) 2023.
   B. ACI STANDARD 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
   C. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402-16).
   D. AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" 360-16.
   E. NDS FOR WOOD CONSTRUCTION WITH 2018 NDS SUPPLEMENT.
   F. ASCE 7-22 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES".
5. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND THE ARCHITECTURAL AND MECHANICAL DRAWINGS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO PERFORMING WORK.
6. IN CASE OF CONFLICT BETWEEN TYPICAL DETAILS, PROJECT SECTIONS, AND/OR DETAILS, THE MOST STRINGENT CONDITION SHALL APPLY.
7. ALL DIMENSIONS MUST BE COORDINATED WITH ARCHITECTURAL DRAWINGS AND WITH EQUIPMENT MANUFACTURER (I.E. WINDOW, DOOR, AIR HANDLER, ETC.). CONTRACTOR MUST OBTAIN AN ARCHITECTURAL DIRECTIVE IN CASE OF ANY CONFLICT. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN IN STRUCTURAL DRAWINGS.
8. ROOFTOP EQUIPMENT ANCHORAGE & OUTDOOR RACK MOUNTED EQUIPMENT ANCHORAGE. ALL ROOF TOP EQUIPMENT CURBS, ROOF TOP MECHANICAL EQUIPMENT, EQUIPMENT TIE DOWNS, AND CONNECTIONS OF ALL EQUIPMENT TO OUTDOOR RACKS OR BUILDING STRUCTURE FOR WIND LOADING ARE TO BE DESIGNED AND ENGINEERED BY A REGISTERED SPECIALTY ENGINEER RETAINED BY THE MECHANICAL EQUIPMENT SUPPLIER. SIGNED AND SEALED DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. THE EQUIPMENT MANUFACTURER SHALL PROVIDE THE ATTACHMENT OF THE UNIT TO THE STRUCTURE AND SUBMIT TO THE ENGINEER LOADS, LOCATIONS, AND METHODS OF ATTACHMENT. THE STRUCTURAL ENGINEER WILL MAKE PROVISIONS IN THE DESIGN OF THE PRIMARY STRUCTURAL FRAME TO ACCOMMODATE THE LOADS AND ATTACHMENTS SUBMITTED BY THE MANUFACTURER.
9. ALLOWANCES FOR THIS PROJECT:
   A. 10 CUBIC YARDS OF 4,000 PSI STRUCTURAL CONCRETE.
   B. 2 TONS REINFORCED STEEL.
   C. 2 TONS STRUCTURAL STEEL FRAMING.
   D. $75,000 ALLOWANCE FOR ADDITIONAL WOOD FRAMING AND CONNECTIONS.
   E. $150,000 ALLOWANCE FOR EXTERIOR RESTORATION.
   F. CONTRACTOR SHALL GIVE CREDIT TO OWNER FOR ANY UNUSED PORTION OF THIS ALLOWANCE AT THE END OF THE PROJECT.

CONCRETE AND REINFORCING:

1. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI-318".
2. ALL CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTHS AS INDICATED BELOW:

CONCRETE STRENGTH	MAX WATER CEMENT RATIO	TYPE AGGREGATE	LOCATION USED
4000 PSI	0.45	STONE	FOUNDATIONS / CONCRETE U.N.O.
3000 PSI	0.52	STONE	SLAB ON GRADE

3. IN CASE A LIGHT WEIGHT MIX IS SPECIFIED, THE STRENGTH AND WATER CEMENT RATIO SHOULD BE AS INDICATED IN THE TABLE ABOVE.
4. STRUCTURAL LIGHT WEIGHT CONCRETE SHOULD HAVE A DENSITY BETWEEN 80 PCF AND 115 PC.
5. ALL REINFORCING STEEL SHALL BE INTERMEDIATE GRADE, NEW BILLET STEEL, DEFORMED BARS, CONFORMING TO ASTM A-615, GRADE 60. ALL BARS SHALL BE SECURELY SUPPORTED AND WIRED IN PLACE. PRIOR TO POURING CONCRETE. ALL REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A-706.
6. ALL WELDED WIRE FABRIC (W.W.F.) IN FLAT SHEETS ONLY AND SHALL CONFORM TO ASTM A-185.
7. UNLESS NOTED, ALL BARS MARKED CONTINUOUS SHALL BE SPLICED AT ALL LAP POINTS AND CORNERS AND DEVELOPED AT NON-CONTINUOUS ENDS AS PER TYPICAL DETAILS. SPLICE CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AND SPLICE CONTINUOUS BOTTOM BARS AT SUPPORTS.
8. CONCRETE COVER FOR REINFORCING BARS SHOWN IN TYPICAL DETAILS.
9. UNLESS NOTED, TEMPERATURE REINFORCING (ASTM A-615-60) TO BE 0.0018 X CONCRETE AREA.
10. PROVIDE #4 @ 12" O.C., WITH STANDARD HOOK, TOP BARS IN ALL SLABS AT DISCONTINUOUS ENDS UNLESS OTHERWISE NOTED ON PLANS. LENGTH OF BARS 1/4 OF SPAN, MINIMUM 3'-0". UNLESS OTHERWISE NOTED PROVIDE #4 @ 12" O.C IN ALL CANTILEVERS. BAR LENGTH SHALL BE CANTILEVER SPAN PLUS 10'-0" PLUS STANDARD HOOK AT CANTILEVER ENDS.
11. WHERE PIPE SLEEVES (UP TO 2" IN DIAMETER) PASS THROUGH CONCRETE BEAMS, PROVIDE ADDITIONAL STIRRUP EACH SIDE OF SLEEVE, SLEEVES FOR PIPES 2" IN DIAMETER OR LARGER MUST BE STEEL OR CAST IRON, AND THE LOCATION MUST BE APPROVED BY THE STRUCTURAL ENGINEER.
12. ALL CONSTRUCTION JOINTS SHALL BE THOROUGHLY CLEANED JUST BEFORE PLACING NEW CONCRETE IN ACCORDANCE WITH THE BUILDING CODE.
13. FOR CHAMFER OF EXPOSED CORNERS OF BEAMS AND/OR COLUMNS, SEE ARCHITECTURAL DRAWINGS.
14. CONTRACTOR SHALL COORDINATE PLACEMENT OF, OR BOX OUT FOR, ALL PIPE SLEEVES, OPENINGS, ETC, REQUIRED FOR VARIOUS TRADES.
15. CONTRACTOR SHALL COORDINATE AND NOTIFY OTHER TRADES IN SUFFICIENT TIME TO ALLOW THEM TO SET ANCHORS, INSERTS, BOLTS, HANGERS, ETC., AS REQUIRED FOR THEIR USE.
16. SEE ARCHITECTURAL DRAWINGS FOR DETAILS OF FLASHING REGLETS, FASCIA DETAILS, ETC.
17. UNDER NO CIRCUMSTANCES SHALL CONCRETE BE PUMPED THROUGH ALUMINUM PIPES. CONCRETE SHALL NOT BE PLACED IN CONTACT WITH ALUMINUM, ALUMINUM MIXING DRUMS, TRUCK MIXERS, BUGGIES, CHUTES, CONVEYORS, TREMIE PIPES, AND OTHER EQUIPMENT MADE OF ALUMINUM SHALL NOT BE USED ON THIS PROJECT.
18. SLUMPS OF OVER 4 INCHES WILL NOT BE PERMITTED UNLESS THE HRWR ADMIXTURE (SUPER PLASTICIZER) IS USED. MAXIMUM SLUMP IS THEN 8 INCHES UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
19. NO ADMIXTURE SHALL BE USED IN CONCRETE EXCEPT WITH THE PERMISSION OF THE ENGINEERS AND AFTER LABORATORY DESIGN MIX APPROVAL. ALL ADMIXTURES SHALL CONTAIN NO MORE CHLORIDE IONS THAN ARE PRESENT IN MUNICIPAL DRINKING WATER.
20. WATER REDUCING ADMIXTURE SHALL CONFORM TO THE ASTM C-494, TYPE A, AND SHALL BE USED IN ALL CONCRETE.
21. AIR ENTRAINING ADMIXTURE SHALL CONFORM TO ASTM C260. AIR CONTENT OF CONCRETE SHALL BE USED AS FOLLOWS:
   A. FOR CONCRETE EXPOSED TO SOIL AND/OR WEATHER, 5%.
   B. FOR INTERIOR WALLS, COLUMNS, AND SLABS, 3%.
22. FLY ASH - ASTMC618, TYPE C OR TYPE F SHOULD BE USED BUT NOT TO EXCEED 20% CEMENTITIOUS CONTENT.
23. ALL EXPOSED CONCRETE SHALL RECEIVE A CURING COMPOUND. THE CURING COMPOUND SHALL CONFORM TO ASTM C309 AND SHALL HAVE 30% SOLIDS MINIMUM. WATER/BLANKET CURING AS PER ACI RECOMMENDATION MAY BE USED AS ALTERNATE.
24. UNLESS NOTED IN PROJECT SPECIFICATIONS, A TESTING LAB SHOULD PERFORM THE FOLLOWING TEST:
   - ATTENDANCE AT THE PROJECT SITE DURING ALL CONCRETE PLACING OPERATIONS
   - CONTROL THE ADDITION OF MIXING WATER TO MAINTAIN THE REQUIRED WATER/CEMENT RATIO AND INDICATED IN THE REPORT ANY ADDED WATER TO THE MIX AND THE LOCATION OF PLACEMENT.
   - ENSURE THAT THE CONCRETE IS OF THE PROPER TEMPERATURE WHEN PLACED.
   - AIR CONTENT TESTS - AT LEAST TWO TESTS SHALL BE MADE FOR EACH DAY'S PLACING OR FROM EACH BATCH OF CONCRETE FROM WHICH CYLINDERS ARE CAST.
   - SLUMP TESTS - AT FREQUENT INTERVALS TO PROPERLY CONTROL THE CONSISTENCY AND AT LEAST ONE AT THE TIME OF CASTING EACH GROUP OF CYLINDERS AND AT LEAST ONE TEST FOR EVERY 25 CUBIC YARDS.
   - CONCRETE COMPRESSION CYLINDERS SHALL BE TAKEN FROM THE CONCRETE OF EACH STRENGTH PLACED ON ANY ONE DAY AT LEAST ONE SET OF FIVE REPRESENTATIVE 6" X 12" TEST CYLINDERS. FOR LARGE PLACEMENTS ON ANY ONE DAY, THERE SHALL BE TAKEN NOT LESS THAN ONE SET OF FIVE REPRESENTATIVE TYPE CYLINDERS FOR EACH 100 CUBIC YARDS OF CONCRETE OF EACH STRENGTH PLACED. TWO CYLINDERS ARE TO BE TESTED AT 7 DAYS, TWO AT THE AGE OF 28 DAYS, AND THE FIFTH CYLINDER IN RESERVE FOR FURTHER TESTING. ASCERTAIN THAT THE TEST SPECIMENS ARE PROPERLY PROTECTED UNTIL SHIPPED TO THE TESTING LABORATORY. RECORD AND IDENTIFY EACH CYLINDER WITH THE LOCATION OF THE CONCRETE FROM WHICH THE SPECIMEN WAS TAKEN. KEEP MARKING IN SEQUENCE.

FOUNDATION NOTES:

1. SITE SOIL FOR THIS PROJECT HAS BEEN INVESTIGATED BY THE FIRM OF GSE ENGINEERING & CONSULTING, INC. AND FOUND, AS PRESENTED IN THEIR REPORT DATED FEBRUARY 20, 2025, SUITABLE TO SUPPORT 2 KSF SPREAD FOOTINGS. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE ABOVE STATED CRITERIA.
2. FILL AND SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER RECOMMENDATION AS CONTAINED IN THEIR REPORT STATED IN ITEM 1.
3. UNDERPINNING SHALL BE PROVIDED TO STABILIZE THE EXISTING FOUNDATIONS AT ALL LOCATIONS AS SHOWN ON PLAN (AT A MINIMUM) IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT. FINAL LAYOUT, SIZE, DEPTH, BRACKET SUPPORTS, ETC. OF ALL UNDERPINNING PILES SHALL BE DESIGNED AND DETAILED BY A DELEGATED ENGINEER AND REVIEWED BY THE STRCTURAL AND GEOTECHNICAL ENGINEER PRIOR TO INSTALLATION.
4. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES UNLESS OTHERWISE NOTED.
5. BACKFILLING AGAINST FOUNDATION WALLS SHALL BE DONE CAREFULLY WITH SMALL COMPACTION EQUIPMENT, AFTER SLABS ON GROUND ARE IN PLACE AND CONCRETE HAS SET. NO TRUCKS, BULLDOZERS, ETC. SHALL BE ALLOWED CLOSER THAN 6'-0" TO ANY FOUNDATION WALL. ANY WALL 3'-0" OR HIGHER MUST BE BRACED DURING THE CONSTRUCTION PROCESS.
6. NO FOUNDATIONS SHALL BE PLACED ABOVE 1 VERTICAL ON 2 HORIZONTAL SLOPES EXTENDED FROM THE CLOSEST EDGE OF ANY UNDISTURBED SOIL OR OTHER FOUNDATION STRUCTURE. BOTTOM OF FOOTINGS SHALL NOT BE LESS THAN 1'-0" BELOW EXISTING GRADE (U.N.O.).
7. FOR FOUNDATIONS SIZE AND REINFORCING SEE SCHEDULE.
8. TERMITE PROTECTION INCLUDING PIPING SLEEVES MUST FOLLOW THE REQUIREMENTS OF SECTIONS 1816.1 AND 1816.2 OF THE FLORIDA BUILDING CODE, 7TH EDITION, 2020.

MASONRY:

1. DESIGN AND CONSTRUCTION SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES TMS 402-16.
2. MINIMUM NET COMPRESSIVE STRENGTH OF BLOCK ASSEMBLY SHALL BE 2000 P.S.I. (f'm) MORTAR FOR MASONRY SHALL BE TYPE "S" OR "M".
3. FOR ALL EXTERIOR AND INTERIOR BEARING, BED JOINTS ARE TO COVER 100% OF THE MASONRY SURFACES AND ALL HEAD JOINTS ARE TO COVER 100% OF THE PROJECTED AREA OF THE FACE SHELLS.
4. FILL ALL CELLS AS REQUIRED WITH 3000 P.S.I. GROUT. SLUMP SHALL BE 8 TO 11 INCHES. SUBMIT DESIGN MIX FOR APPROVAL.
5. MINIMUM HORIZONTAL JOINT REINFORCING SHALL BE 9 GAUGE HOT DIP GALVANIZED TRUSS OR LADDER TYPE JOINT REINFORCING AT 16" O.C., PROVIDE MANUFACTURE "T" AND "L" SHAPES FOR INTERSECTIONS AND CORNERS, (MINIMUM LAP 6").
6. MINIMUM VERTICAL REINFORCING SHALL BE (1)-#5 @ 48" OR (1)-#4 @ 32" O.C., (U.N.O.).
7. PROVIDE ADDITIONAL VERTICAL REINFORCING BAR AT EVERY CORNER, INTERSECTION, CONTROL JOINT, AND OPENING EDGES (U.N.O.).
8. MINIMUM SPLICE FOR VERTICAL REINFORCING IS SHOWN IN DETAIL 4-023, SPLICE FOR HORIZONTAL JOINT REINFORCING = 6".
9. WALLS ARE DESIGNED TO BE BRACED BY FLOOR OR ROOF MEMBERS, CONTRACTOR SHALL PROVIDE TEMPORARY BRACING DURING CONSTRUCTION.
10. ALL CELLS BELOW FIRST FLOOR FINISHED ELEVATION MUST BE FULLY GROUT FILLED.
11. ALL KNOCK OUT BLOCK HORIZONTAL BARS SHALL HAVE CORNER BARS AT ALL CORNERS AND WALL INTERSECTIONS. SIZE AND NUMBER OF CORNER BARS SHALL BE SAME AS HORIZONTAL BARS.
12. ALL INTERSECTING WALLS AND CORNER WALLS SHALL BE LAID IN AN OVERLAPPING MASONRY BONDING PATTERN, WITH ALTERNATE UNITS HAVING A BEARING OF NOT LESS THAN 3 INCHES ON UNIT BELOW.

STRUCTURAL STEEL:

1. ALL STRUCTURAL STEEL WORK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST A.I.S.C. SPECIFICATIONS.
2. STRUCTURAL STEEL SHALL CONFORM TO:

WIDE FLANGE (WF)	ASTM A992 (50 KSI)
SHAPES (L,T,C,PL)	ASTM A36
STRUCTURAL TUBE (HSS)	ASTM A500 (46 KSI)
STEEL PIPE (HSS)	ASTM A500 (42 KSI)
ANCHOR BOLTS	ASTM F1554 (36 KSI) U.N.O. IN PLANS, OR SECTIONS.
FRAMING BOLTS	ASTM A325 OR A490
SHEAR STUDS	ASTM A108
WELDING ELECTRODES	E70XX

3. ALL HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM SPECIFICATION A325 AND SHALL BE PROVIDED WITH HARDENED WASHERS UNDER THE TURNED ELEMENT (NUT OR BOLT HEAD).
4. INSTALLATION AND TIGHTENING OF ALL HIGH STRENGTH BOLTS SHALL CONFORM TO THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
5. SHOP CONNECTIONS MAY BE WELDED OR HIGH STRENGTH BOLTED. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM. ALL CONNECTIONS SHALL CONFORM TO THE TYPICAL CONNECTION DETAILS SHOWN ON THE PLANS UNLESS SPECIFICALLY APPROVED BY THE ENGINEER.
6. ALL FIELD CONNECTIONS SHALL BE BOLTED WITH HIGH STRENGTH BOLTS, SLIP-CRITICAL (FRICTION) TYPE EXCEPT WHERE SLOTTED HOLES ARE SPECIFIED OR WHERE MOVEMENT OF THE CONNECTED MEMBERS IS EXPECTED. IN THESE CASES PROVIDE OVERSIZED WASHER, HAND TIGHTEN BOLTS, AND TACK WELD WASHER TO NUT TO VERIFY ASSEMBLY IS HELD TOGETHER.
7. ALL WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY CODE, ANS01.1, ALL WELDING SHALL BE PERFORMED USING E70XX U.N.O.
8. CUTS, HOLES, COPINGS, ETC. REQUIRED IN STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES SHALL BE SHOWN IN THE STRUCTURAL STEEL SHOP DRAWINGS AND SHALL BE MADE IN THE SHOP. HOLES SHALL BE REINFORCED AS REQUIRED BY THE ENGINEER.
9. BURNING OF HOLES, CUTS, ETC. IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED, EXCEPT WITH THE SPECIFIC APPROVAL OF THE ENGINEER.
10. ALL STEEL MEMBERS (SUCH AS BEAMS, COLUMNS, PLATES, BOLTS, LINTELS, DOOR JAMBS, ETC.) EXPOSED TO WEATHER AND ALL MEMBERS IN AN EXTERIOR ENCLOSED NON-AIR-CONDITIONED AREA, SHALL BE HOT DIPPED GALVANIZED.
11. FOR MISCELLANEOUS STEEL, SEE ARCHITECTURAL DRAWINGS.
12. ANY STEEL MEMBERS REQUIRED BY THE ELECTRICAL OR MECHANICAL TRADES FOR THE SUPPORT OF THEIR EQUIPMENT, WHICH ARE NOT SHOWN ON ARCHITECTURAL OR STRUCTURAL DRAWINGS, SHALL BE PROVIDED BY THE TRADE REQUIRING SUCH SUPPORT.
13. SEE SPECIFICATIONS FOR PAINTING OF STRUCTURAL STEEL. ALL FABRICATION AND ERECTION MARKS SHALL BE COVERED DURING FIELD TOUCH-UP PAINTING.
14. ALL CONNECTIONS TO BE DOUBLE ANGLE FRAMED BEAM CONNECTION PER AISC UNLESS NOTED OTHERWISE. ALL BOLTS TO BE 3/4" MINIMUM DIAMETER UNLESS NOTED OTHERWISE. SHOP CONNECTIONS MAY BE WELDED OR BOLTED. WELDS ARE TO BE EQUAL IN STRENGTH TO BOLTS.
15. DESIGN CONNECTIONS FOR HALF THE MAXIMUM SHEAR (V IN KIPS) LISTED IN THE TABLE 3-6 "MAXIMUM TOTAL UNIFORM LOAD" AT THE BOTTOM OF EACH PAGE IN THE "BEAM PROPERTIES" OF THE AISC 360-16" MANUAL OF STEEL CONSTRUCTION. "MINIMUM CONNECTION SHALLCONSIST OF TWO 3/4"Ø A325 BOLTS. REACTIONS SHOWN ARE BASED ON UNFACTORED LOADS. PROVIDE SIGNED AND SEALED DRAWINGS AND CALCULATIONS BY A PROFESSIONAL ENGINEER.
16. DESIGN BASE PLATE ANCHOR BOLTS FOR LATERAL MEMBERS USING FORCES INDICATED IN DRAWINGS. PROVIDE SIGNED AND SEALED DRAWINGS AND CALCULATIONS BY A PROFESSIONAL ENGINEER.
17. WHEN STEEL MEMBERS ARE WELDED TO EMBED PLATES IN CONCRETE, WELDING PROCESS SHOULD BE PERFORMED IN SUCH WAY THAT EMBED PLATE DOES NOT OVERHEAT AND EXPAND. SUCH EXPANSION WILL CRACK THE CONCRETE SURROUNDING THE EMBED PLATE AND MAY WEAKEN THE STRUCTURAL CAPACITY OF THE CONNECTION. WE RECOMMEND TO PROVIDE SEVERAL SINGLE PASSES TO BUILT UP THE WELD SIZE REQUIRE WITH COOLING OFF PERIODS TO AVOID THE EMBED PLATE EXPANSION. UNDER NO CIRCUMSTANCES PROVIDE MORE THAN 6" OF 1/4" WELD WITHOUT ALLOWING A COOLING OFF PERIOD.
18. EXPOSED ENDS OF STRUCTURAL TUBES OR PIPES SHALL BE CAPPED WITH A MINIMUM 1/4" PLATE U.N.O.
19. ALL WELDS SHALL BE VISUALLY INSPECTED AND COMPLIANT WITH AWS D1.1 ACCEPTANCE CRITERIA. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE TESTED FOR THEIR FULL LENGTH BY MEANS OF AN APPROVED NONDESTRUCTIVE TEST IN ACCORDANCE WITH AWS D1.1.

PURPOSE OF DRAWING STATEMENT:

THESE STRUCTURAL DRAWINGS REPRESENT A LEVEL OF COMPLETION EQUIVALENT TO APPROXIMATELY 90% CONSTRUCTION DOCUMENTS AND DO NOT REPRESENT A COMPLETED STRUCTURAL DESIGN. ANY BIDDING OR PROCUREMENT BASED ON THESE DRAWINGS WILL BE DONE AT THE CONTRACTOR'S RISK AND MUST BE ACCOMPANIED BY APPROPRIATE CONTRACTOR CONTINGENCIES TO ACCOUNT FOR THE UNCOMPLETED NATURE OF THE DOCUMENTS AND EVENTUAL CHANGES AS THE DESIGN PROGRESSES.

STEEL JOIST:

1. STEEL JOIST CONSTRUCTION SHALL CONFORM TO THE LATEST SPECIFICATIONS OF, AND THE JOISTS SHALL BE APPROVED BY, THE STEEL JOIST INSTITUTE.
2. UNLESS OTHERWISE NOTED, EACH SHORT SPAN JOISTS MINIMUM OF 2-1/2" ON STEEL BEAMS AND BEAR LONG SPAN JOIST MINIMUM OF 4" ON STEEL SUPPORTS. IN CASES WHERE JOISTS BEAR ON BEAMS FROM ONE SIDE ONLY, JOIST SEATS SHALL EXTEND A MINIMUM OF 1" PAST THE CENTERLINE OF SUPPORTING BEAM.
3. ALL JOISTS BEARING ON BEAMS SHALL BE WELDED OR BOLTED TO THOSE BEAMS.
4. PROVIDE BRIDGING FOR ALL JOISTS AS SHOWN ON PLAN BUT NOT LESS THAN WHAT IS REQUIRED BY THE STEEL JOIST INSTITUTE OR THE STEEL JOIST DESIGNER. BRIDGING SHALL CONSIST OF MINIMUM 1-1/4" X 1-1/4" X 7/64" ANGLES, (U.N.O.).
5. ALL BRIDGING SHALL BE PROVIDED AND INSTALLED BY JOIST SUPPLIER.
6. ALL CLIPS AND CONNECTIONS SHALL BE SHOP WELDED.
7. NO FIELD WELDING TO BAR JOISTS EXCEPT ITEMS SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS SHALL BE ALLOWED WITHOUT SPECIFIC PERMISSION FROM THE ENGINEER.
8. NO JOIST SHALL BE FIELD SPLICED.
9. MAXIMUM DEFLECTION OF STEEL JOISTS = L/360 FOR LIVE LOAD.
10. FOR PAINTING OF STEEL JOIST, SEE SPECIFICATIONS.
11. JOIST MANUFACTURER SHALL SUBMIT WITH THE SHOP DRAWINGS HIS CATALOG USED FOR THE MANUFACTURE OF JOISTS, INDICATING THE LOAD TABLES AND SIZES OF ALL MEMBERS USED.
12. NO LOADS EXCEEDING 40 POUNDS MAY BE HUNG FROM JOISTS WITHOUT SPECIFIC PERMISSION FROM THE STRUCTURAL ENGINEER. LOADS LESS THAN 40 POUNDS MAY BE HUNG AT PANEL POINTS ONLY. ANY COST INVOLVED IN REINFORCING OF JOISTS SHALL BE BORNE BY THE PRIME CONTRACTOR REQUIRING ADDED LOADS.
13. ALL SHORT SPAN JOISTS, AND DEEP LONG SPAN JOISTS SHALL HAVE UNIFORM CROSS SECTION, WITH STANDARD DEAD LOAD CAMBER. ROOF PITCH IS ACCOMPLISHED BY SLOPED JOISTS AND SUPPORT BEAMS. ADJUST JOIST SEATS AS SHOWN ON DRAWINGS.
14. FOR SPECIFIC JOIST ENDS, SEE ROOF SECTIONS.
15. RIGID CONNECTIONS OF BOTTOM CHORDS OF JOISTS TO COLUMNS SHALL BE MADE ONLY AFTER THE APPLICATION OF ALL THE DEAD LOADS. PROVIDE LOOSE BOLTED CONNECTION OF THESE BOTTOM CHORDS DURING ERECTION.
16. JOIST SHALL NOT BE FABRICATED USING ELECTRICAL RESISTANCE WELDING.

STEEL ROOF DECK:

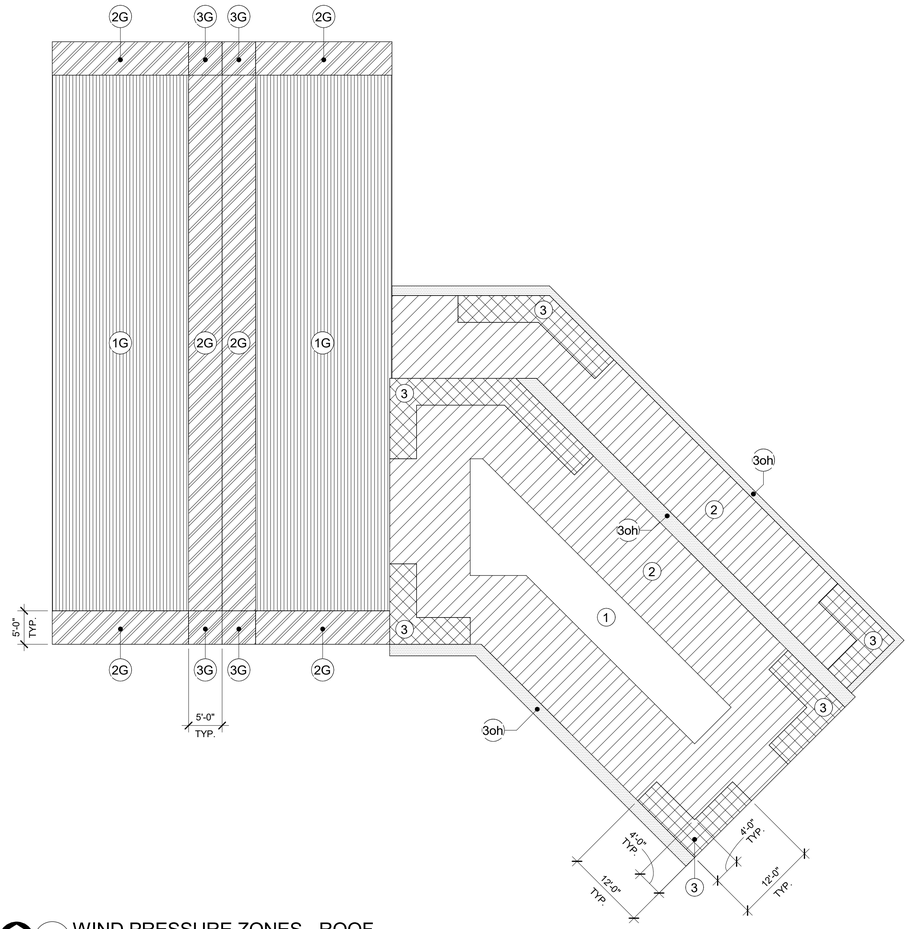
1. STEEL ROOF DECK SHALL BE A MINIMUM OF 1-1/2" - 20 GAUGE WIDE RIB FOR SPANS UP TO 6'-0" OR 3" TYPE N - 20 GAUGE FOR SPANS UP TO 12'-0" . THE SIZE, TYPE AND GAUGE INDICATED ABOVE SHOULD BE USED UNLESS A DIFFERENT ONE IS INDICATED IN THE ROOF FRAMING NOTES OF THE ROOF PLAN DRAWING.
2. ALL STEEL ROOF DECK SHALL BE GALVANIZED G90 AS PER ASTM SPECIFICATIONS. ALL ROOF DECK SHOULD BE VENTED WHEN A LIGHT WEIGHT INSULATING CONCRETE IS USED FOR ROOFING.
3. ALL STEEL ROOF DECK SHALL BE CAPABLE OF SUPPORTING ALL CONSTRUCTION LOADS.
4. ALL STEEL ROOF DECK SHALL BE CONTINUOUS OVER FOUR OR MORE STRUCTURAL SUPPORTS (I.E. DECK SHOULD BE DETAILED FOR A THREE SPAN CONDITION.)
5. STEEL ROOF DECK SHALL HAVE NESTING SIDE LAPS (ATTACHED BY MECHANICAL MEANS). PROVIDE FASTENER LAYOUT AS PER TYPICAL DETAIL 5-305.
6. IF DECK IS CUT IN SINGLE SPAN CONDITION, EACH END OF SUCH SECTIONS SHALL BE WELDED TO ITS SUPPORT THROUGH WELDING WASHERS IN THE BOTTOM OF EACH RIB.
7. IN AREAS WHERE THE DECK IS CUT AS PER NOTE 6, THE GAUGE OF THE SINGLE SPAN DECK SHALL BE ADJUSTED UPWARDS AS REQUIRED BY THE ENGINEER TO SUPPORT THE LOADS.
8. ANY ELECTRICAL WORK WEIGHING MORE THAN 5 PSF OR 50 LBS CONCENTRATED SHALL BE HUNG FROM STEEL BEAMS/JOISTS ONLY. FOR HANGERS, SEE SPECIFICATIONS. ALL MECHANICAL WORK AND PIPING SHALL BE HUNG FROM STEEL BEAMS/JOISTS.
9. METAL DECK CONTRACTOR TO PROVIDE 18 GAUGE RIDGE PLATE, VALLEY PLATE, EDGE STRIP, ETC., AS REQUIRED.
10. STEEL ROOF DECK SHALL BE WELDED AT ENDS AND ALL INTERMEDIATE SUPPORTING MEMBERS WITH 5/8" DIAMETER PUDDLE WELDS OR ELONGATED WELDS OF EQUAL STRENGTH SPACED PER SPECIFICATIONS IN THE BOTTOM OF THE RIB ACROSS THE WIDTH OF THE DECK UNIT.
11. CUT OUT METAL DECK WHERE BOLT PROJECTIONS INTERFERE WITH METAL DECK.
12. DIRECTION OF METAL DECK SHOWN THUS ◄——► ON PLAN.

LIGHTGAUGE METAL FRAMING:

1. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE, "SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS", 2016 EDITION. PROVIDE SIGN AND SEALED CALCULATIONS AND DRAWINGS FOR ALL LIGHT GAUGE STRUCTURAL ELEMENTS OF THE BUILDING, INCLUDING THE EXTERIOR METAL STUDS (CURTAIN WALL), AND ALL EXTERIOR CEILINGS.
2. ALL STRUCTURAL STUDS AND JOISTS 22, 20, AND 18 GAUGES SHALL BE FORMED FROM GALVANIZED STEEL PER ASTM A653, G60 COATING MEETING THE REQUIREMENTS OF ASTM C955 WITH A YIELD STRENGTH OF 33,000 PSI.
3. ALL STRUCTURAL STUDS AND JOISTS 16, 14, AND 12 GAUGES SHALL BE FORMED FROM GALVANIZED STEEL PER ASTM A653, G60 COATING MEETING ASTM C955, WITH YIELD STRENGTH OF 50,000 PSI.
4. ALL STRUCTURAL TRACK AND BRIDGING SHALL BE FORMED FROM GALVANIZED STEEL PER ASTM A653, G60 COATING MEETING THE REQUIREMENTS OF ASTM C595, WITH YIELD STRENGTH OF 33,000 PSI.
5. WITH EACH TYPE OF METAL FRAMING REQUIRED, PROVIDE MANUFACTURER'S STANDARD STEEL RUNNERS (TRACKS), BLOCKING, LINTELS, CLIP ANGELS, SHOES, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY MANUFACTURER FOR APPLICATIONS INDICATED, AS NEEDED TO PROVIDE A COMPLETE METAL FRAMING SYSTEM.
6. PROVIDE GALVANIZED FINISH TO METAL FRAMING COMPONENTS COMPLYING WITH ASTM A653 FOR MINIMUM COATING DESIGNATIONS. ATTACH SIMILAR COMPONENTS BY WELDING. ATTACH DISSIMILAR COMPONENTS BY WELDING, BOLTING OR SCREW FASTENERS, AS STANDARD WITH MANUFACTURER. ALL WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED AND EXPERIENCED IN LIGHT GAUGE STRUCTURAL STEEL FRAMING WORK.
7. INSTALL METAL FRAMING SYSTEMS IN ACCORDANCE WITH MANUFACTURER'S PRINTED OR WRITTEN INSTRUCTIONS AND RECOMMENDATIONS, UNLESS OTHERWISE INDICATED.
8. INSTALL CONTINUOUS TRACKS SIZED TO MATCH STUDS.
9. SET STUDS PLUMB, EXCEPT AS NEEDED FOR DIAGONAL BRACING OR REQUIRED FOR NON-PLUMB WALLS OR WARPED SURFACED AND SIMILAR REQUIREMENTS.
10. WHERE STUD SYSTEM ABUTS STRUCTURAL COLUMN OR WALLS, INCLUDING MASONRY WALL, ANCHOR ENDS OF STIFFENERS TO SUPPORTING STRUCTURE.
11. SECURE STUDS TO TOP AND BOTTOM RUNNER TRACKS BY EITHER WELDING OR SCREW FASTENERS AT BOTH INSIDE AND OUTSIDE FLANGES.

DEMOLITION AND REPAIR NOTES:

1. PROVIDE AND MAINTAIN SHORING, BRACING OR STRUCTURAL SUPPORT TO PRESERVE STABILITY AND PREVENT MOVEMENT, SETTLEMENT, OR COLLAPSE OF CONSTRUCTION TO REMAIN, AND TO PREVENT UNEXPECTED OR UNCONTROLLED MOVEMENT OR COLLAPSE OF CONSTRUCTION BEING DEMOLISHED.
2. PROCEED WITH SELECTIVE DEMOLITION SYSTEMATICALLY, FROM HIGHER TO LOWER LEVEL. COMPLETE SELECTIVE DEMOLITION OPERATIONS ABOVE EACH FLOOR OR TIER BEFORE DISTURBING SUPPORTING MEMBERS ON THE NEXT LOWER LEVEL.
3. NEATLY CUT OPENINGS AND HOLES PLUMB, SQUARE AND TRUE TO DIMENSIONS REQUIRED. USE CUTTING METHODS LEAST LIKELY TO DAMAGE CONSTRUCTION TO REMAIN OR ADJOINING CONSTRUCTION. USE HAND TOOLS OR SMALL POWER TOOLS DESIGNED FOR SAWING OR GRINDING, NOT HAMMERING OR CHOPPING, TO MINIMIZE DISTURBANCE OF ADJACENT SURFACES. TEMPORARILY COVER OPENINGS TO REMAIN.
4. REMOVE STRUCTURAL FRAMING AND LOWER STRUCTURAL MEMBERS TO GROUND BY METHOD SUITABLE TO AVOID FREE FALL AND TO PREVENT GROUND IMPACT OR DUST GENERATION.
5. LOCATE SELECTIVE DEMOLITION EQUIPMENT AND REMOVE DEBRIS AND MATERIALS SO AS NOT TO IMPOSE EX



1 WIND PRESSURE ZONES - ROOF
3/32" = 1'-0"

LOAD SCHEDULE:

ROOF: DEAD LOAD

ROOFING & INSULATION	=	10 PSF
METAL DECK	=	3 PSF
FRAMING	=	5 PSF
M/E/P	=	7 PSF
CEILING & MISC.	=	5 PSF
		30 PSF
LIVE LOAD	=	20 PSF
		50 PSF

WIND DESIGN DATA:

CODE:
FLORIDA BUILDING CODE, 2023 (8th Ed.) ASCE 7-22

BASIC WIND SPEED	135 mph (Vult) 105 mph (Vasd)
CATEGORY (RISK)	III
EXPOSURE	B
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	± 0.18

* GLAZED OPENINGS IN RISK CATEGORY II, III, IV LOCATED IN HURRICANE PRONE REGIONS SHALL BE PROTECTED IN ACCORDANCE WITH FBC 2023 SEC. 1609.1.2

COMPONENTS AND CLADDING (ULTIMATE) UPLIFT PRESSURE SCHEDULE

PATTERN	ZONE	GABLE ROOF					
		EFFECTIVE WIND AREA					
		10 SQ. FT.	20 SQ. FT.	50 SQ. FT.	100 SQ. FT.	200 SQ. FT.	500 SQ. FT.
	1G	+22 PSF/-47 PSF	+20 PSF/-42 PSF	+18 PSF/-36 PSF	+16 PSF/-32 PSF	+16 PSF/-27 PSF	+16 PSF/-27 PSF
	2G	+22 PSF/-74 PSF	+20 PSF/-64 PSF	+18 PSF/-49 PSF	+16 PSF/-39 PSF	+16 PSF/-39 PSF	+16 PSF/-39 PSF
	3G	+22 PSF/-88 PSF	+20 PSF/-75 PSF	+18 PSF/-57 PSF	+16 PSF/-44 PSF	+16 PSF/-44 PSF	+16 PSF/-44 PSF

COMPONENTS AND CLADDING (ULTIMATE) UPLIFT PRESSURE SCHEDULE

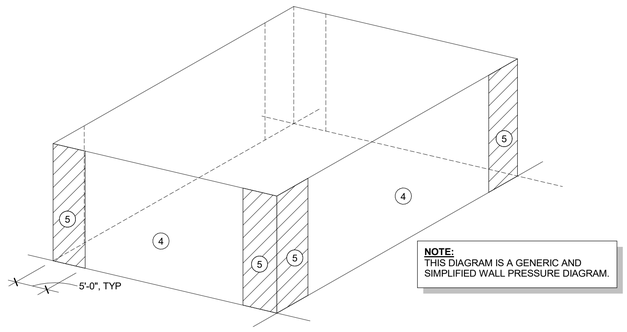
PATTERN	ZONE	FLAT ROOF					
		EFFECTIVE WIND AREA					
		10 SQ. FT.	20 SQ. FT.	50 SQ. FT.	100 SQ. FT.	200 SQ. FT.	500 SQ. FT.
	1'	+17 PSF/-38 PSF	+16 PSF/-38 PSF	+16 PSF/-38 PSF	+16 PSF/-38 PSF	+16 PSF/-33 PSF	+16 PSF/-26 PSF
	1	+17 PSF/-66 PSF	+16 PSF/-61 PSF	+16 PSF/-56 PSF	+16 PSF/-51 PSF	+16 PSF/-47 PSF	+16 PSF/-41 PSF
	2	+17 PSF/-86 PSF	+16 PSF/-81 PSF	+16 PSF/-74 PSF	+16 PSF/-68 PSF	+16 PSF/-63 PSF	+16 PSF/-55 PSF
	3	+17 PSF/-118 PSF	+16 PSF/-107 PSF	+16 PSF/-92 PSF	+16 PSF/-81 PSF	+16 PSF/-70 PSF	+16 PSF/-55 PSF
	1oh	-66 PSF	-65 PSF	-63 PSF	-62 PSF	-55 PSF	-45 PSF
	2oh	-86 PSF	-79 PSF	-69 PSF	-62 PSF	-55 PSF	-45 PSF
	3oh	-118 PSF	-105 PSF	-88 PSF	-75 PSF	-62 PSF	-45 PSF

COMPONENTS AND CLADDING
DESIGN WIND PRESSURE (ULTIMATE) FOR WALLS, DOORS & WINDOWS

PATTERN	ZONE	EFFECTIVE WIND AREA					
		10 SQ. FT.	20 SQ. FT.	50 SQ. FT.	100 SQ. FT.	200 SQ. FT.	500 SQ. FT.
	4	+38 PSF/-41 PSF	+36 PSF/-39 PSF	+34 PSF/-37 PSF	+32 PSF/-36 PSF	+31 PSF/-34 PSF	+29 PSF/-32 PSF
	5	+38 PSF/-50 PSF	+36 PSF/-47 PSF	+34 PSF/-43 PSF	+32 PSF/-39 PSF	+31 PSF/-36 PSF	+29 PSF/-32 PSF

NOTE:

1. ALL EXTERIOR DOORS & WINDOW ASSEMBLIES SHALL SATISFY THE REQUIREMENTS OF THE FLORIDA BUILDING CODE (EIGHTH EDITION 2023, SECTION 1709.5). ALL CONNECTIONS TO BUILDING STRUCTURE SHALL HAVE THE CAPACITY TO WITHSTAND THE PRESSURES INDICATED ON SCHEDULES.
2. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.
3. ALL WIND PRESSURE VALUES INDICATED ARE IN POUNDS PER SQUARE FOOT (PSF). MULTIPLY ULTIMATE WIND PRESSURE BY 0.60 FOR ASD PRESSURE.
4. FOR NET UPLIFT CALCULATION, THE WEIGHT OF THE STRUCTURAL MEMBER AND THE STRUCTURAL DECK SUPPORTED ARE THE ONLY TWO LOADS THAT CAN BE DEDUCTED FROM THE UPLIFT PRESSURES INDICATED ON SCHEDULES.



NOTE:
THIS DIAGRAM IS A GENERIC AND SIMPLIFIED WALL PRESSURE DIAGRAM.

WIND PRESSURE ZONES - WALLS
SCALE: 1/8" = 1'-0"

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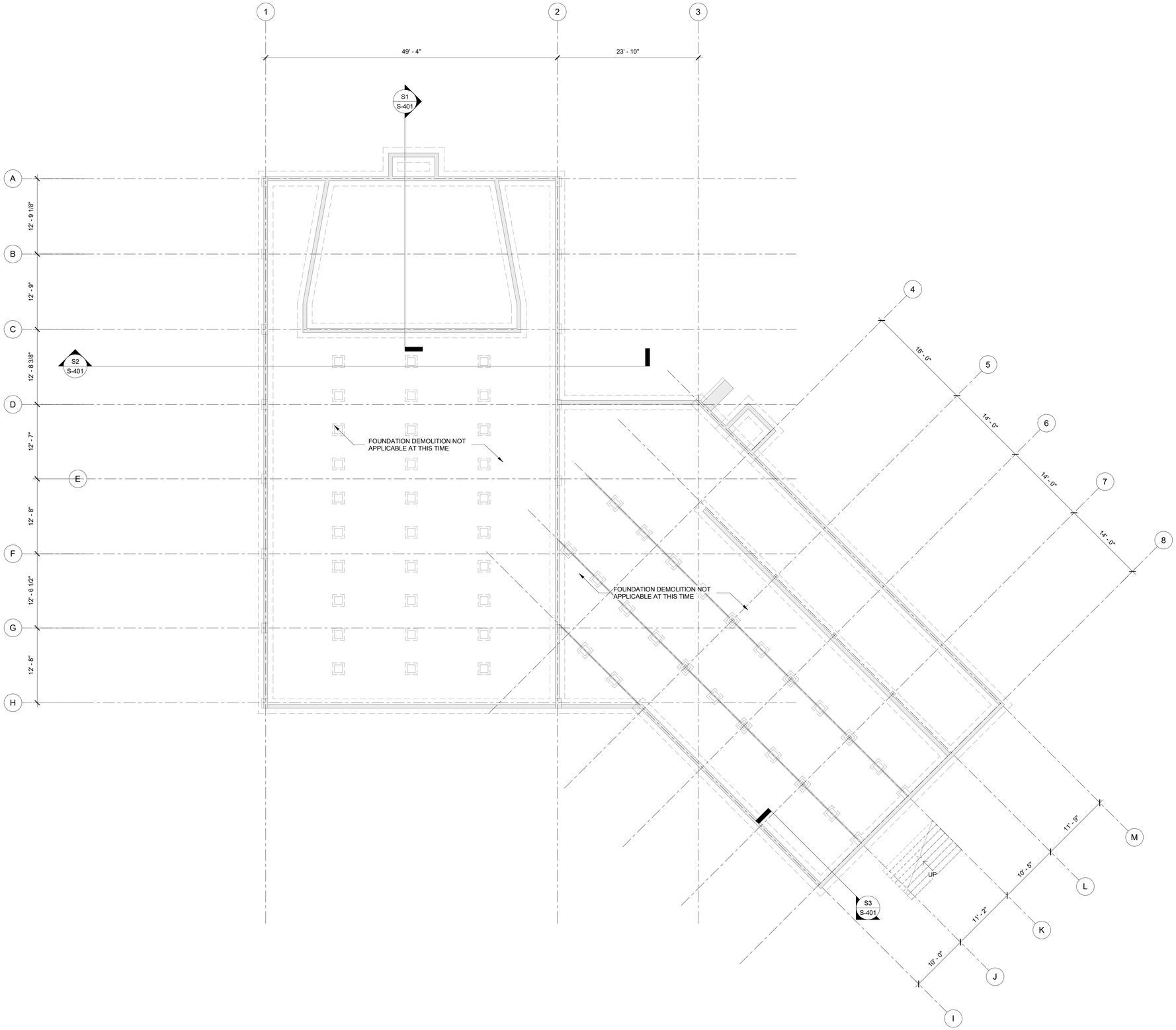
WIND DESIGN DATA AND LOAD SCHEDULE

S-102



1 FOUNDATION DEMOLITION PLAN
1/8" = 1'-0"

DEMOLITION NOTES:

1. PROVIDE AND MAINTAIN SHORING, BRACING OR STRUCTURAL SUPPORT TO PRESERVE STABILITY AND PREVENT MOVEMENT, SETTLEMENT, OR COLLAPSE OF CONSTRUCTION TO REMAIN, AND TO PREVENT UNEXPECTED OR UNCONTROLLED MOVEMENT OR COLLAPSE OF CONSTRUCTION BEING DEMOLISHED.
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6. NEW OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS ARE NOT PERMITTED.
7. CONTRACTOR SHALL REMOVE ALL EXISTING INTERIOR CEILINGS, FINISHES, ETC. TO EXPOSE THE EXISTING STRUCTURAL FRAMING. NOTIFY THE ENGINEER OF RECORD IMMEDIATELY ONCE THE EXISTING STRUCTURE IS EXPOSED FOR FINAL REVIEW AND DETERMINATION OF REPAIRS.

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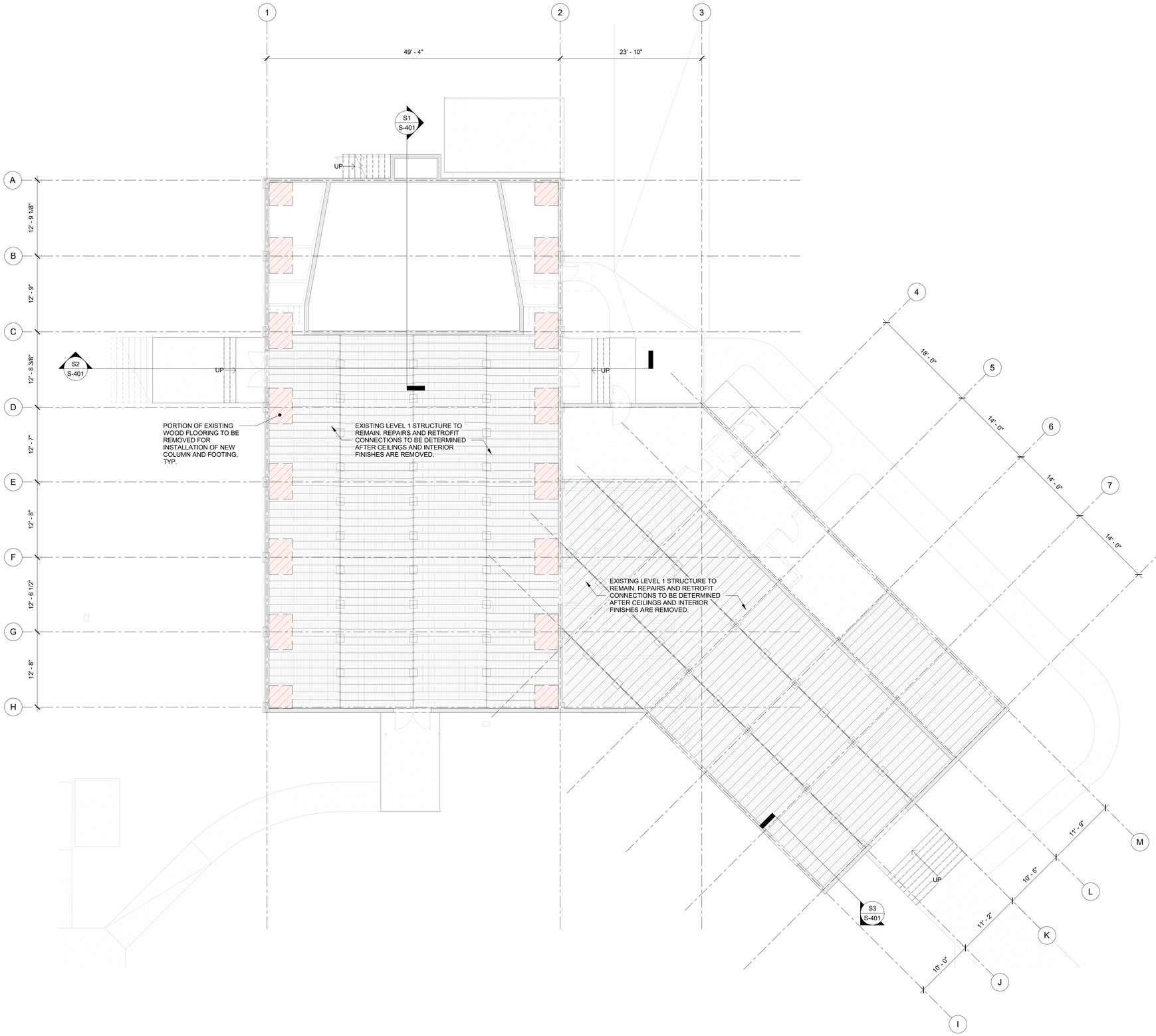
FOUNDATION DEMOLITION PLAN

S-103



1 LEVEL 1 DEMOLITION PLAN
1/8" = 1'-0"

DEMOLITION NOTES:

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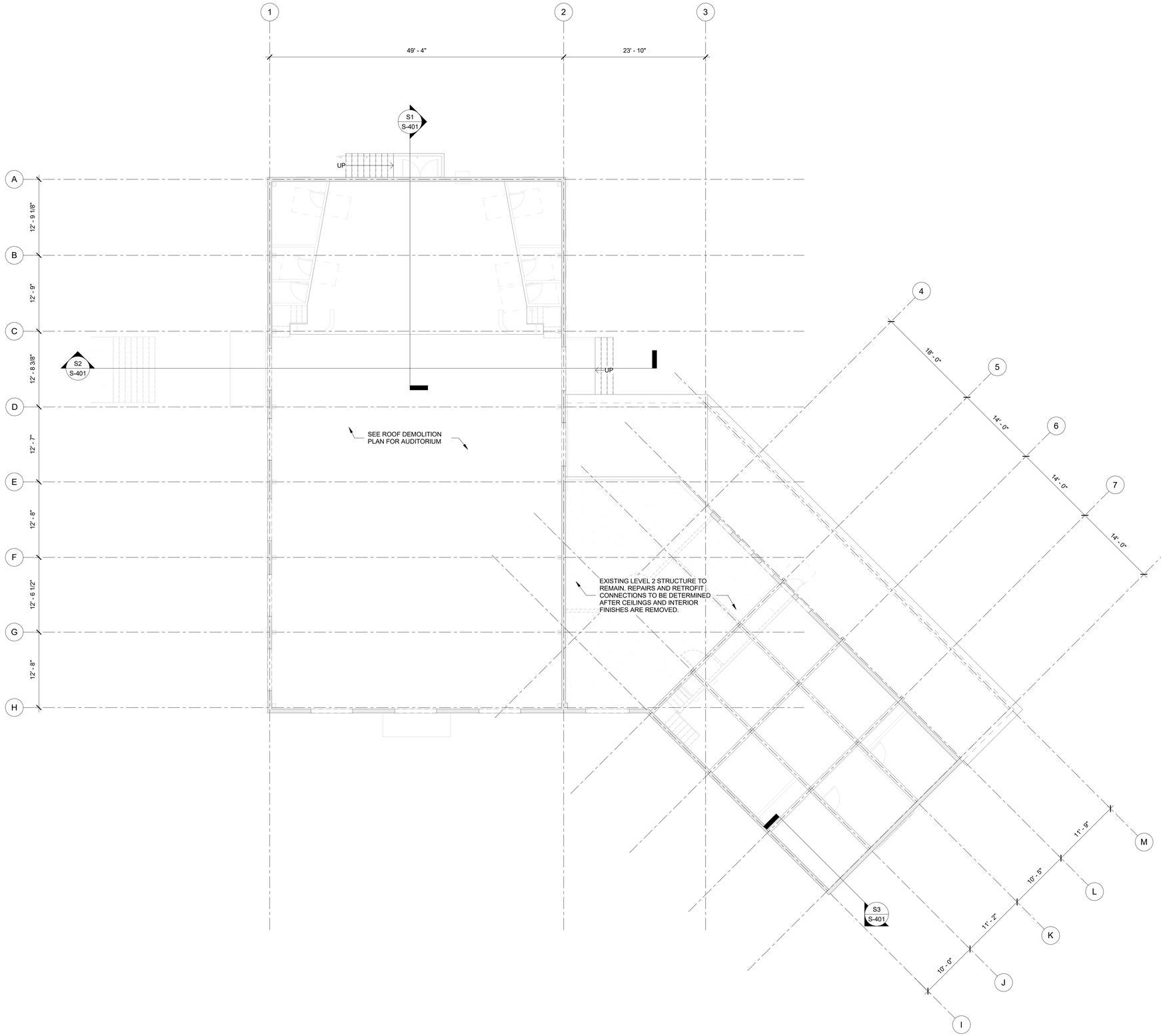
LEVEL 1 DEMOLITION PLAN

S-104



1 LEVEL 2 DEMOLITION PLAN
1/8" = 1'-0"

DEMOLITION NOTES:

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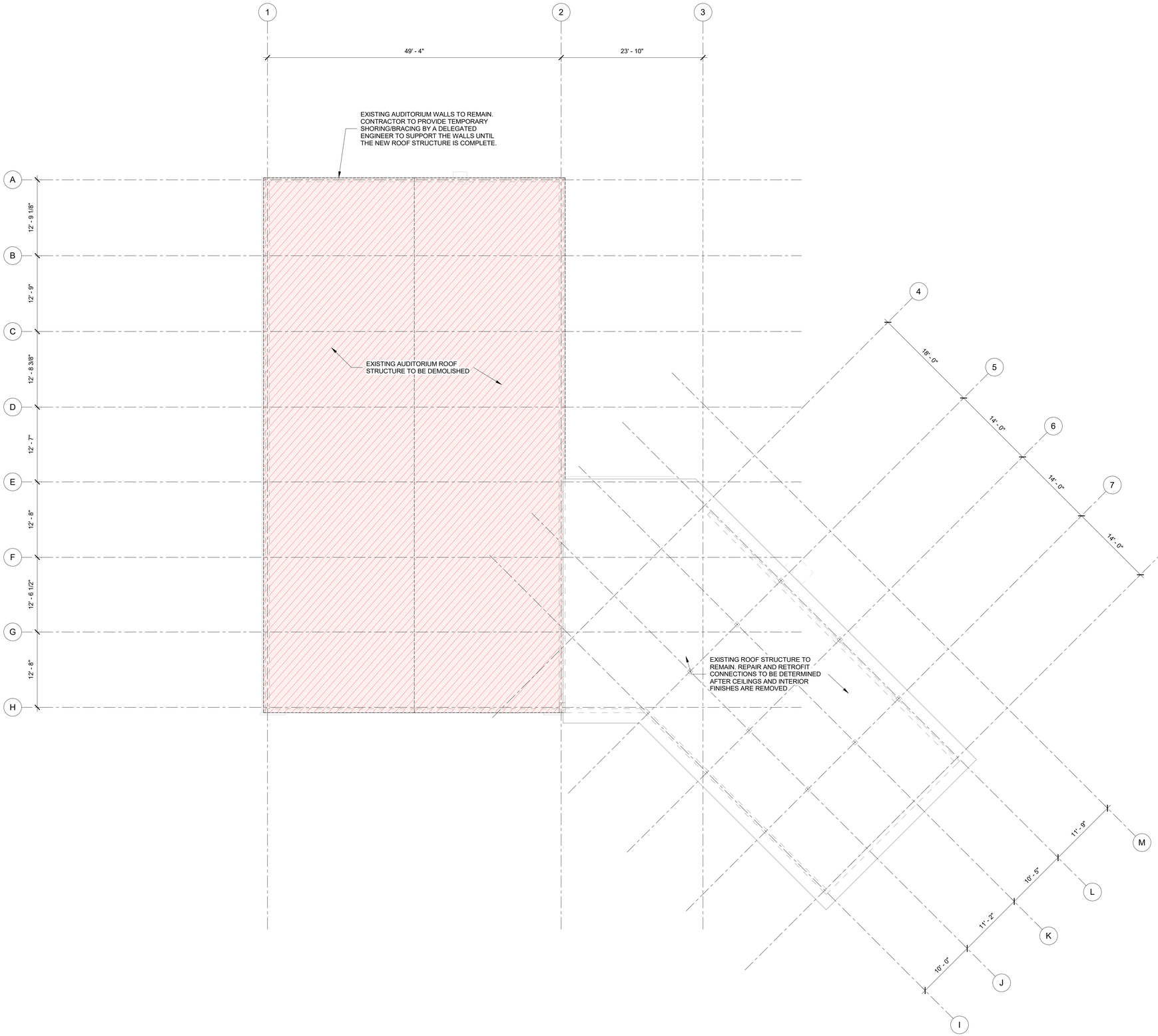
LEVEL 2 DEMOLITION PLAN

S-105



1 ROOF DEMOLITION PLAN
1/8" = 1'-0"

DEMOLITION NOTES:

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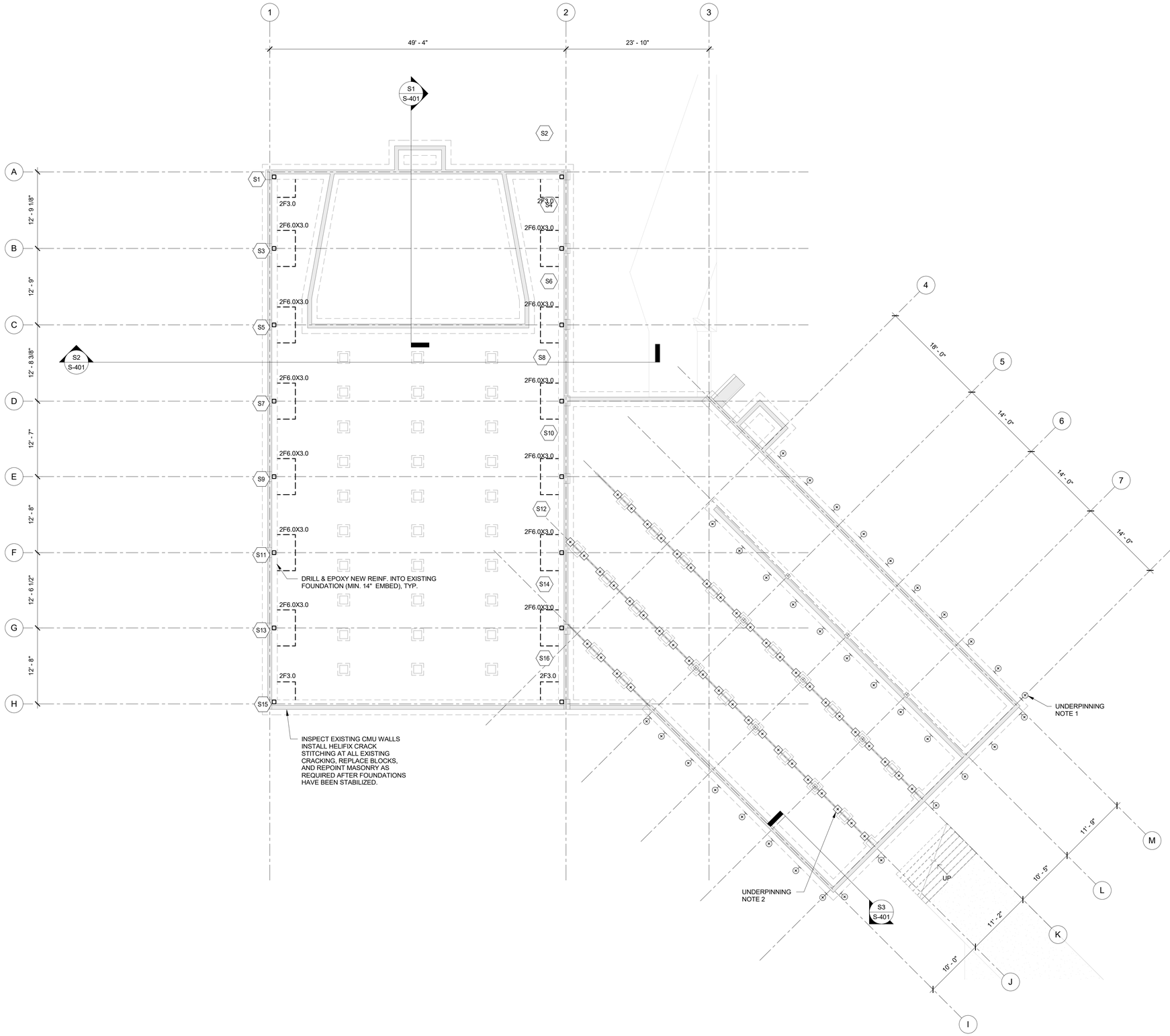
ROOF DEMOLITION PLAN

S-106



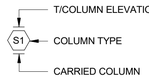
1 FOUNDATION PLAN
1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. EXISTING FOUNDATIONS CONSITS OF SHALLOW SPREAD COLUMN FOOTINGS AND CONTINUOUS WALL FOOTINGS.
2. FOR TYPICAL FOUNDATION PLAN DETAILS, SEE S-3 SERIES DRAWINGS.
3. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-101.
4. FOR FOOTING SIZE AND REINFORCING, SEE SCHEDULE ON THIS DRAWING.
5. ( ) DENOTES TOP OF FOOTING ELEVATION.
6. CONTRACTOR TO FIELD VERIFY TOP OF EXISTING FOUNDATION ELEVATIONS. TOP OF NEW FOUNDATIONS SHALL MATCH TOP OF EXISTING FOUNDATIONS, U.N.O.
7. FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
8. TYPICAL REINFORCING FOR EXISTING WALLS:

   A. REINFORCE EXISTING CMU WALLS W/ (1) #5 VERTICAL @ 32" O.C. TYPICAL FOR ALL WALLS, U.N.O. SEE TYPICAL DETAIL 4-201 / S-302 FOR PROCEDURE TO REINFORCE EXISTING CMU WALLS.
   B. ▦ INDICATES ADDITIONAL (1) VERTICAL IN GROUT FILLED CELL.
   C. PROVIDE ADDITIONAL (1) VERTICAL IN FIRST (2) CELLS EACH SIDE OF ALL WALL OPENINGS, CORNERS AND INTERSECTIONS.

9. COLUMN DESIGNATION SHOWN THUS ⬡ ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-601.



10. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN WITH DOWELS TO MATCH, U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".

COLUMN FOOTING SCHEDULE

MARK	LENGTH	WIDTH	THICKNESS*	TOP & BTM REINF.* EA. WAY U.N.O.	REMARKS
2F3.0	3' - 0"	3' - 0"	1' - 0"	#5@10" O.C	STD. ACI HOOK
2F6.0X3.0	6' - 0"	3' - 0"	1' - 0"	#5@10" O.C	STD. ACI HOOK

* THICKNESS SHOWN IN SCHEDULE IS MIN., MATCH EXISTING FNDN THICKNESS, U.N.O.

UNDERPINNING NOTES:

1. INDICATES NEW UNDERPINNING PILES AND SUPPORT BRACKET BY DELEGATED ENGINEER AT CONTINUOUS WALL FOOTINGS, COORD. W/ GEOTECH REPORT.
2. INDICATES NEW UNDERPINNING PILES AND SUPPORT BRACKET BY DELEGATED ENGINEER AT BUILT-UP 2X SUPPORT GIRDERS EA. SIDE OF EXIST. CMU PIERS, TYP. COORD. W/ GEOTECH REPORT.
   *ALTERNATE:PROVIDE NEW UNDERPINNING PILE AT MID-SPAN OF SUPPORT GIRDER AND ABANDON EXISTING CMU PIERS. SISTER NEW MEMBERS AT EXISTING GIRDERS DUE TO HINGE CREATED AT CMU PIER SUPPORT CONDITIONS.

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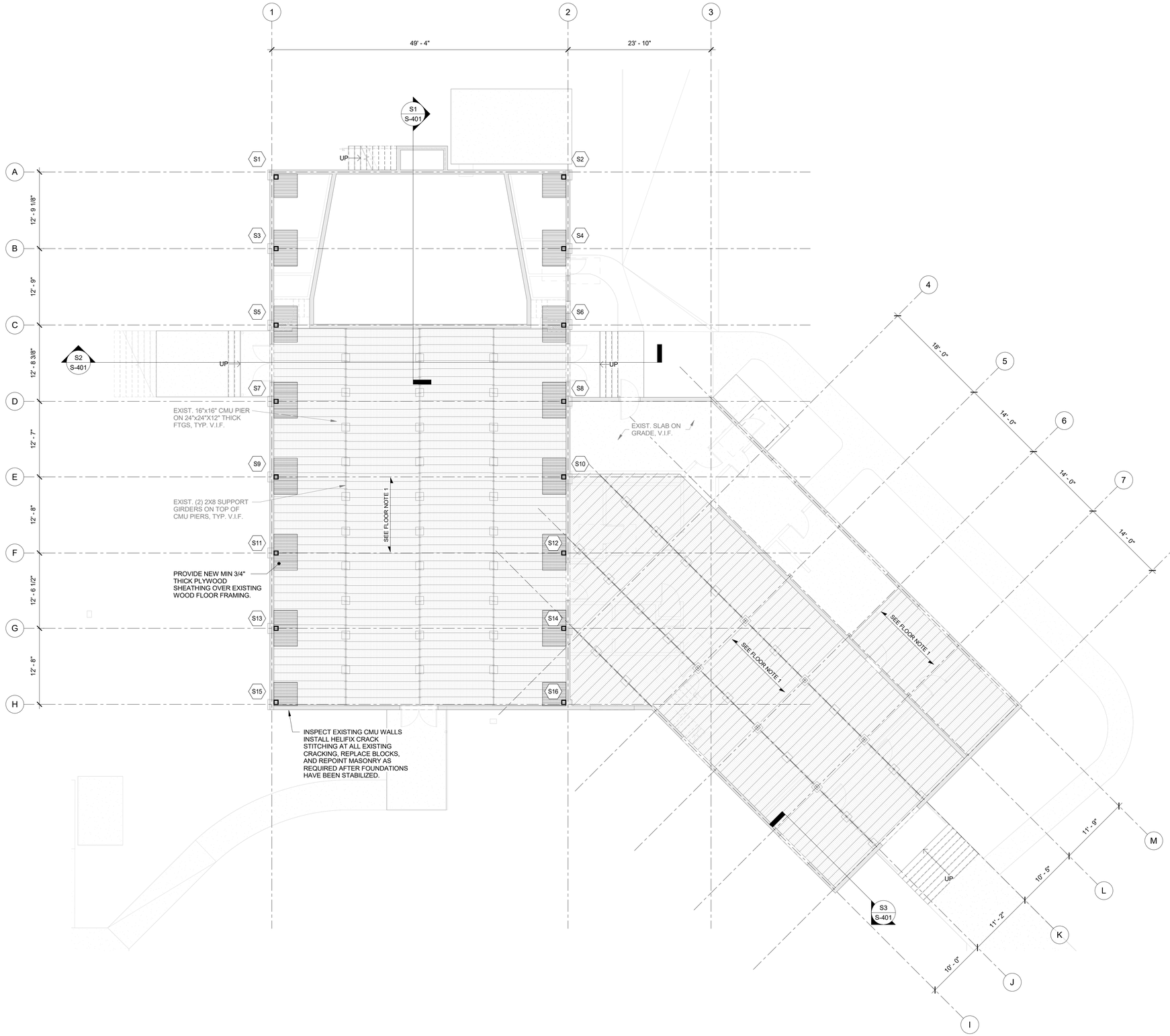
FOUNDATION PLAN

S-201



1 LEVEL 1 PLAN
1/8" = 1'-0"

FLOOR FRAMING NOTES:

1. EXISTING FLOOR FRAMING CONSISTS OF APPROXIMATELY 3/4" THICK WOOD PLANKS SPANNING OVER 2X WOOD JOISTS AT 16" O.C. SUPPORTED BY BUILT-UP 2X BEAMS ON CMU PILASTERS.
2. EXISTING SCHEMATIC STRUCTURAL FRAMING SHOWN ON THIS PLAN HAS BEEN DEVELOPED FROM LIMITED SITE INSPECTIONS. EXISTING STRUCTURE TO BE FIELD VERIFIED AS REQUIRED.
3. CONTRACTOR TO NOTIFY ENGINEER ONCE ALL CEILING AND INTERIOR FINISH DEMOLITION IS COMPLETE AND EXISTING STRUCTURE IS FULLY EXPOSED TO VERIFY EXISTING FRAMING CONDITIONS.
4. DIRECTION OF WOOD DECK SPAN SHOWN THUS ⟶ ON PLAN.
5. CONTRACTOR TO FIELD VERIFY EXISTING TOP OF FLOOR ELEVATION, APPROXIMATE TOP OF FLOOR REFERENCE ELEVATION SHOWN THUS ⟨T.F. = ⟩ ON PLAN, FOR ACTUAL EL., SEE CIVIL DRAWINGS.
6. FOR TYPICAL FOUNDATION PLAN DETAILS, SEE S-3 SERIES DRAWINGS.
7. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-101.
8. FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
9. COLUMN DESIGNATION SHOWN THUS ⬡ ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-601.
10. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN WITH DOWELS TO MATCH, U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
11. NEW MASONRY CONTROL JOINTS SHOWN THUS ◀ MCJ ON PLAN (IF SHOWN) SHALL BE PROVIDED IN EXISTING CMU WALLS. MAXIMUM SPACING OF JOINTS = 24' - 0", COORDINATE LOCATION WITH THE ARCH. & STRUCT WALL ELEVATIONS. DO NOT LOCATE A MCJ CLOSER THAN 24" TO ANY CMU OPENING. SEE TYPICAL DETAIL 4-002 / S-302.
12. CONTRACTOR TO INSPECT AND REPLACE ANY DAMAGED WOOD MEMBERS SHOWING MORE THAN 10% SECTION LOSS DUE TO TERMITES, FUNGAL DECAY, SPLITTING OR MOISTURE DAMAGE. NEW MEMBERS OF SAME SIZE AND LENGTH CAN BE SISTERED ONTO EXISTING MEMBERS USING SOUTHERN YELLOW PINE NO. 2 OR BETTER.

WALL SCHEDULE

TYPE	WALL THICKNESS	TYPE OF WALL	WALL REINF.	REMARKS

THICKENED SLAB SCHEDULE

MARK*	SIZE	THICKNESS	TOP & BTM* REINF. CONT.	TOP & BTM* REINF. TRANSV.	REMARKS

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PRELIMINARY NOT FOR CONSTRUCTION

THELMA BOLTIN CENTER

CITY OF GAINESVILLE
516 NE 2nd Ave, Gainesville, FL 32601

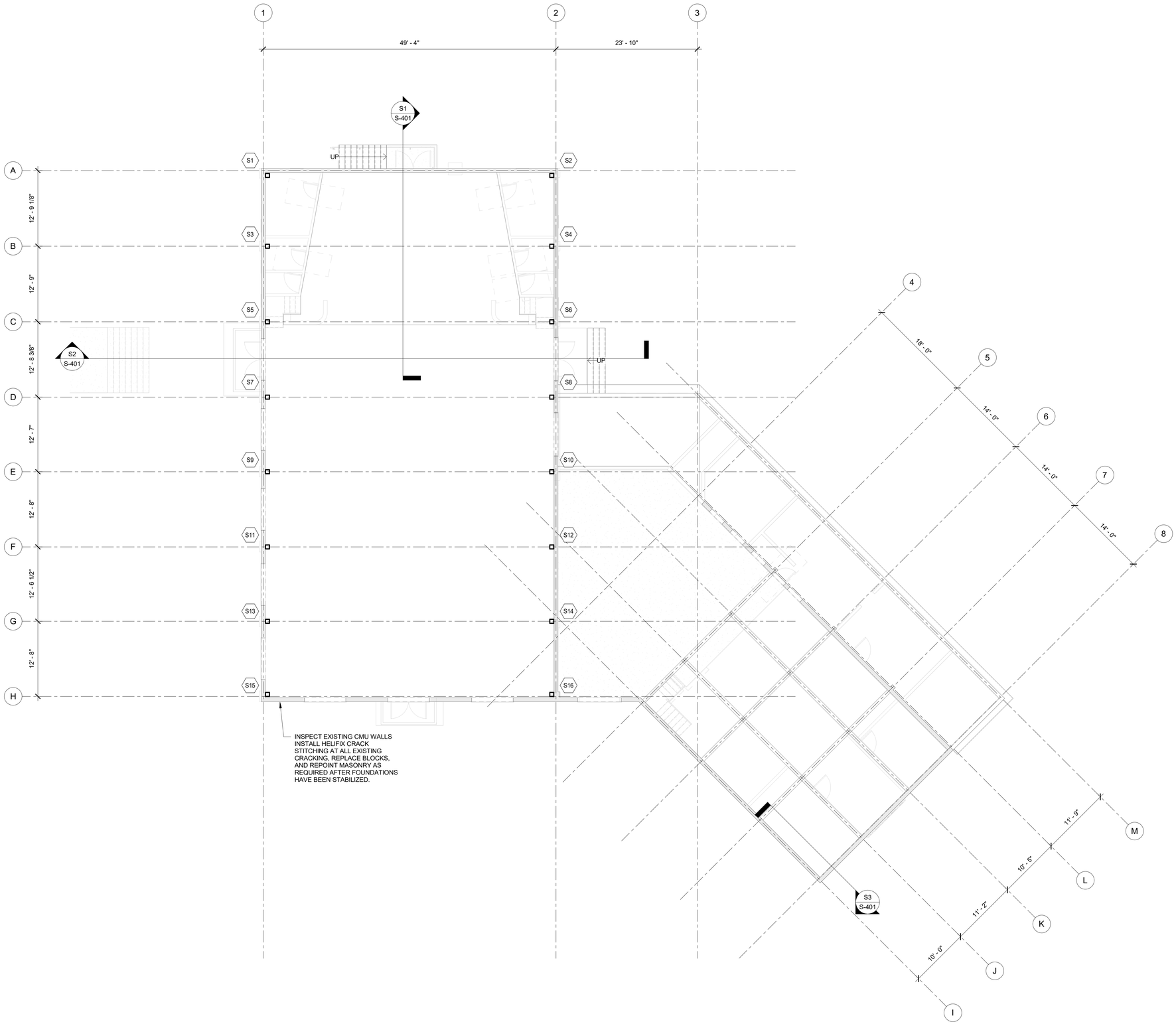
Project number
2018

90% Review Set 04/11/2025

REV	DESCRIPTION	DATE

LEVEL 1 PLAN

S-202



1 2ND FLOOR FRAMING PLAN
1/8" = 1'-0"

FLOOR FRAMING NOTES:

1. EXISTING FLOOR FRAMING CONSISTS OF APPROXIMATELY 3/4" THICK WOOD PLANKS SPANNING OVER 2X WOOD JOISTS AT 16" O.C. SUPPORTED BY BUILT-UP 2X BEAMS ON CMU PILASTERS.
2. EXISTING SCHEMATIC STRUCTURAL FRAMING SHOWN ON THIS PLAN HAS BEEN DEVELOPED FROM LIMITED SITE INSPECTIONS. EXISTING STRUCTURE TO BE FIELD VERIFIED AS REQUIRED.
3. CONTRACTOR TO NOTIFY ENGINEER ONCE ALL CEILING AND INTERIOR FINISH DEMOLITION IS COMPLETE AND EXISTING STRUCTURE IS FULLY EXPOSED TO VERIFY EXISTING FRAMING CONDITIONS.
4. DIRECTION OF WOOD DECK SPAN SHOWN THUS ◄——► ON PLAN.
5. CONTRACTOR TO FIELD VERIFY EXISTING TOP OF FLOOR ELEVATION, APPROXIMATE TOP OF FLOOR REFERENCE ELEVATION SHOWN THUS [T.F. =] ON PLAN, FOR ACTUAL EL., SEE CIVIL DRAWINGS.
6. FOR TYPICAL FOUNDATION PLAN DETAILS, SEE S-3 SERIES DRAWINGS.
7. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-101.
8. FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
9. COLUMN DESIGNATION SHOWN THUS ⬡ ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-601.
10. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN WITH DOWELS TO MATCH, U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
11. NEW MASONRY CONTROL JOINTS SHOWN THUS ◄| MCJ ON PLAN (IF SHOWN) SHALL BE PROVIDED IN EXISTING CMU WALLS. MAXIMUM SPACING OF JOINTS = 24' - 0", COORDINATE LOCATION WITH THE ARCH. & STRUCT WALL ELEVATIONS. DO NOT LOCATE A MCJ CLOSER THAN 24" TO ANY CMU OPENING. SEE TYPICAL DETAIL 4-002 / S-302.
12. CONTRACTOR TO INSPECT AND REPLACE ANY DAMAGED WOOD MEMBERS SHOWING MORE THAN 10% SECTION LOSS DUE TO TERMITES, FUNGAL DECAY, SPLITTING OR MOISTURE DAMAGE. NEW MEMBERS OF SAME SIZE AND LENGTH CAN BE SISTERED ONTO EXISTING MEMBERS USING SOUTHERN YELLOW PINE NO. 2 OR BETTER.

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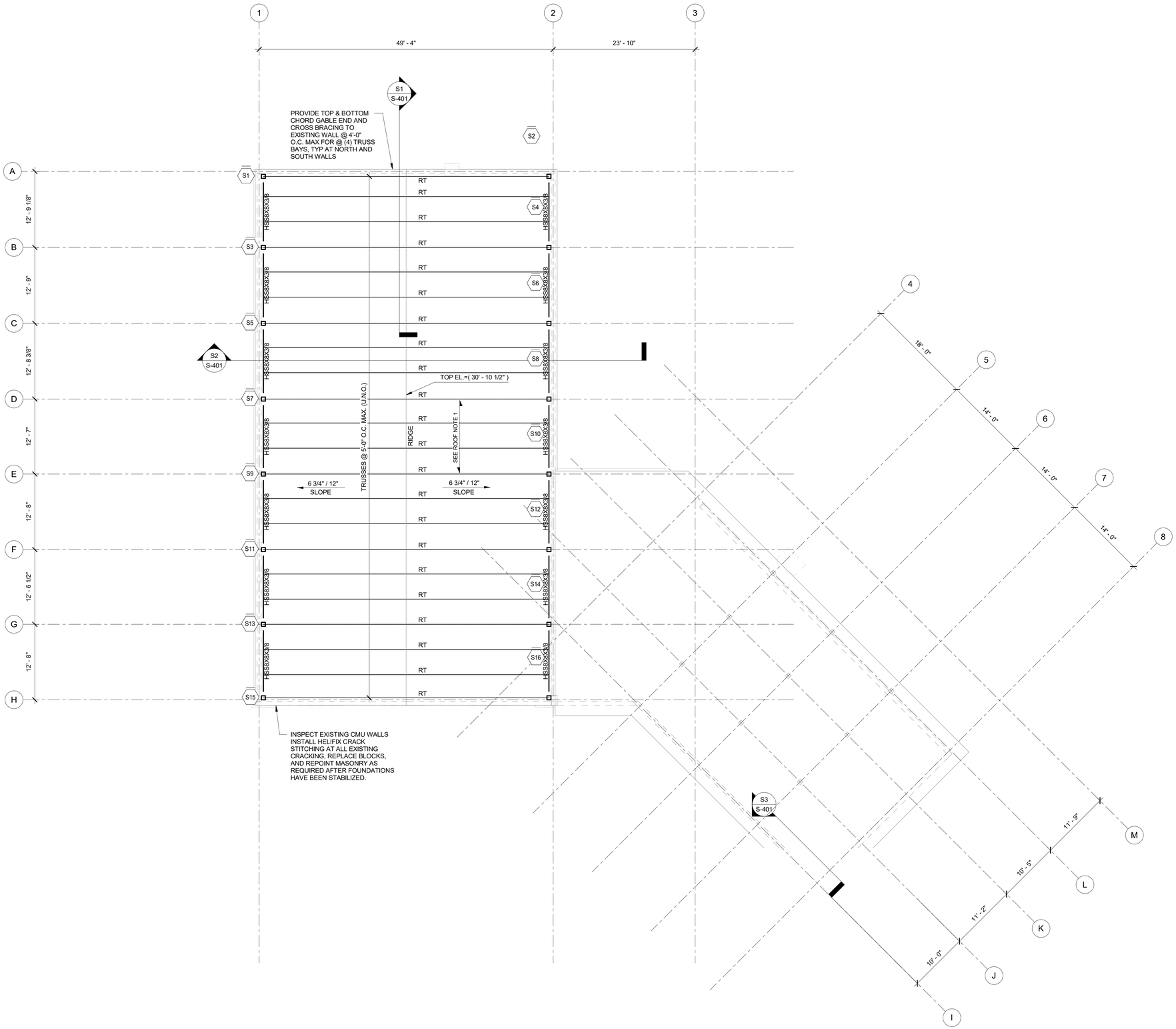
LEVEL 2 PLAN

S-203



1 AUDITORIUM ROOF FRAMING PLAN
1/8" = 1'-0"

ROOF FRAMING NOTES (AUDITORIUM):

1. ROOF FRAMING SHALL BE 1-1/2" - 20 GAUGE TYPE "B" (WIDE RIB) 80 KSI YIELD STRENGTH, GALVANIZED G90 METAL DECK SPANNING OVER PRE-ENGINEERED, PRE-FABRICATED METAL TRUSSES SPACED @ 5'-0" O.C. MAX. (U.N.O.). REFER TO ARCHITECTURAL DRAWINGS FOR ROOF SLOPES AND FINISH.
2. DIRECTION OF METAL DECK SPAN SHOWN THUS ◄——► ON PLAN.
3. FOR TYPICAL FOUNDATION PLAN DETAILS, SEE S-3 SERIES DRAWINGS.
4. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-101.
5. ( ) DENOTES UNDERSIDE OF METAL DECK (U.M.D.) ELEVATION.
6. UNLESS OTHERWISE NOTED, FASTEN ALL STRUCTURAL ELEMENTS IN ACCORDANCE WITH THE LATEST FLORIDA BUILDING CODE.
7. DELEGATED TRUSS DESIGNER TO DESIGN TRUSSES FOR THE UPLIFT FORCES SHOWN ON THE LOADING SCHEDULE AND PROVIDE CERTIFICATION THAT SHORD AND WEB MEMBERS HAVE BEEN DESIGNED FOR LOAD REVERSAL STRESSES DUE TO UPLIFT FORCES.
8. TRUSS MANUFACTURER SHALL HAVE THE OPTION TO CHANGE LOCATION OF ROOF TRUSSES IF REQUIRED. SUBMIT TRUSS SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA FOR REVIEW AND APPROVAL.
9. LOCATION OF PERMAMENT AND TEMPORARY TRUSS BRACING IS NOT SHOWN AND SHALL BE DETERMINED AND DETAILED BY THE TRUSS DESIGN DELEGATED ENGINEER. ALL BRACING SHALL BE SHOWN ON THE TRUSS DRAWINGS.
10. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS DESIGN DELEGATED ENGINEER.
11. COORDINATE ALL TRUSS MEMBERS WITH DUCTWORK, MECHANICANAL, AND ARCHITETURAL DRAWINGS.
12. COORDINATE ROOF SHAPE, DIMENSIONS, AND SLOPE WITH ARCHITECTURAL DRAWINGS.
13. FOR PLAN DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
14. COLUMN DESIGNATIONS SHOWN THUS ⬡ ON PLAN. FOR COLUMN SCHEDULE, SEE DRAWING S-601.
15. ⬡ DENOTES COLUMN STOP AT THIS LEVEL.
16. CONTRACTOR TO INSPECT AND REPLACE ANY DAMAGED WOOD MEMBERS SHOWING MORE THAN 10% SECTION LOSS DUE TO TERMITES, FUNGAL DECAY, SPLITTING OR MOISTURE DAMAGE. NEW MEMBERS OF SAME SIZE AND LENGTH CAN BE SISTERED ONTO EXISTING MEMBERS USING SOUTHERN YELLOW PINE NO. 2 OR BETTER W/ 2 ROWS OF 10d NAILS AT 12" O.C.

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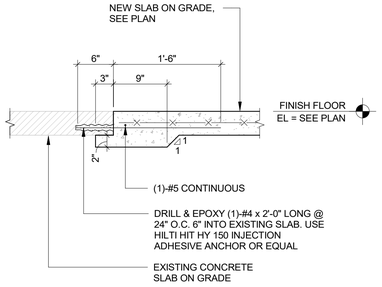
ROOF FRAMING PLAN

S-204



NEW SLAB TO EXISTING SLAB SECTION

3-011

f'c = 4000 PSI, NORMAL WEIGHT

BAR SIZE	LAP CLASS	TENSION LAP SPLICES				COMPRESSION LAP SPLICES
		LAP LENGTH PER SPACING AND COVER CASE				
		CASE 1		CASE 2		
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	
#3	A B	19 24	15 19	28 36	22 28	12
#4	A B	25 32	19 25	37 48	29 37	15
#5	A B	31 40	24 31	47 60	36 47	19
#6	A B	37 48	29 37	56 72	43 56	23
#7	A B	54 70	42 54	81 106	63 81	26
#8	A B	62 80	48 62	93 121	71 93	30
#9	A B	70 91	54 70	105 136	81 105	34
#10	A B	79 102	61 79	118 153	91 118	38
#11	A B	87 113	67 87	131 170	101 131	42

NOTES:

CASE 1

BEAMS AND COLUMNS:
CONCRETE COVER >/= TO BAR DIAMETER, C-C BAR SPACING >/= TO 2X BAR DIAMETER AND WITH STIRRUPS OR TIES THROUGHOUT TENSION LAP SPLICE LENGTH NOT LESS THAN THE CODE MINIMUM.

OTHER MEMBERS:
CONCRETE COVER >/= TO THE BAR DIAMETER AND C-C BAR SPACING >/= TO 3X BAR DIAMETER.

CASE 2

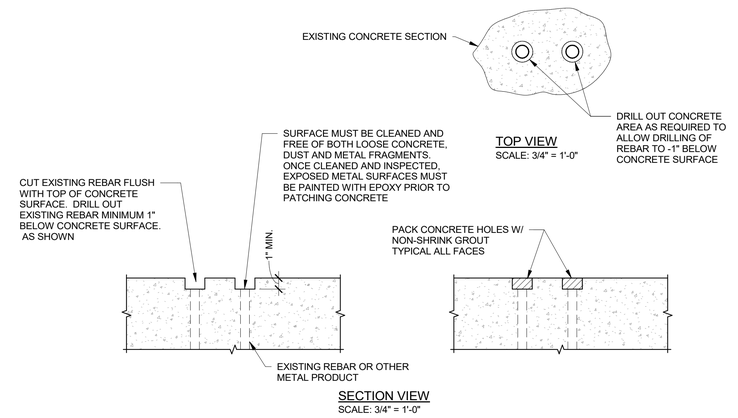
BEAMS AND COLUMNS:
CONCRETE COVER < BAR DIAMETER AND C-C BAR SPACING < 2X BAR DIAMETER.

OTHER MEMBERS:
CONCRETE COVER < BAR DIAMETER OR C-C BAR SPACING < 3X BAR DIAMETER.

* LAP CLASS "B" IS TO BE USED UNLESS OTHERWISE SPECIFIED IN THESE CONTRACT DOCUMENTS.

TENSION AND COMPRESSION LAP SPLICES WITH F'c = 4000 PSI

3-066



EXISTING EMBEDDED REBAR CUT OFF PROTECTION DETAIL

3-206

NOTE:
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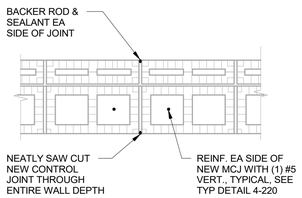
TYPICAL DETAILS

S-301

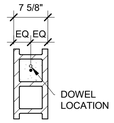


NOTE:
1. SEE PLAN FOR NEW MASONRY CONTROL JOINTS PROVIDED IN EXISTING CMU WALLS. MAXIMUM SPACING OF JOINTS = 24' - 0". COORDINATE LOCATION WITH THE ARCH. & STRUCT WALL ELEVATIONS. DO NOT LOCATE A MCJ CLOSER THAN 24" TO ANY CMU OPENING.

NEW MASONRY CONTROL JOINT IN EXISTING WALL

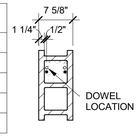
4-002

LAP SPLICE SCHEDULE FOR SINGLE REINFORCED 8" CMU WALL		
BAR SIZE	LAP SPLICE	REMARKS
#3	18"	
#4	24"	
#5	30"	
#6	36"	



LAP SPLICE SCHEDULE SINGLE REINFORCED 8" CMU WALL

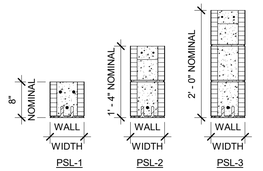
LAP SPLICE SCHEDULE FOR DOUBLE REINFORCED 8" CMU WALL		
BAR SIZE	LAP SPLICE	REMARKS
#3	18"	
#4	24"	
#5	30"	
#6	36"	



LAP SPLICE SCHEDULE DOUBLE REINFORCED 8" CMU WALL

4-023

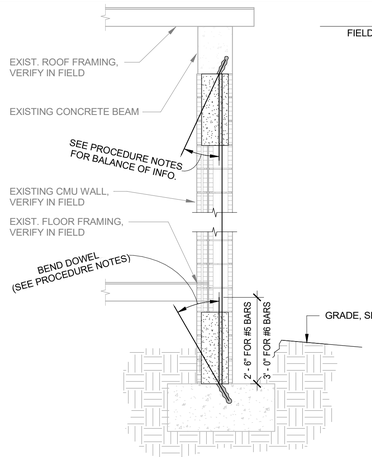
POWERS STEEL LINTEL SCHEDULE					
WALL THICKNESS	LINTEL DEPTH	BOTTOM REINFORCING	TOP REINFORCING	STIRRUPS & SPACING	OPENING SIZE
PSL-1	8"	(1)-#5	(2)-#5	-	UP TO 4'-0"
PSL-2	16"	(1)-#5	(2)-#5	-	4'-0" TO 7'-0"
PSL-3	24"	(1)-#5	(2)-#5	-	7'-0" TO 10'-0"



NOTES:
1. POWERS STEEL LINTELS SHALL BE RETROFIT INSTALLED AT ALL EXISTING OPENINGS PER SCHEDULE.
2. THE FIRST VERTICAL CELL EACH OF THE OPENING SHALL BE GROUTED AND CONTAIN (1) #5 VERTICAL.
3. MASONRY CONTROL JOINT SHALL NOT BE LOCATED WITHIN WIDTH OF OPENING, NOR SHALL IT BE LOCATED WITHIN 24" OF THE OPENING ON EITHER SIDE.
4. POWERS STEEL LINTELS SHALL BE INSTALLED PER MANUFACTURER SPECIFICATIONS.
5. CONTRACTOR SHALL PROVIDE ALL NECESSARY SHORING TO SUPPORT THE CMU WALL LOAD ABOVE FOR LINTEL INSTALLATION.

TYPICAL POWERS STEEL LINTEL SCHEDULE

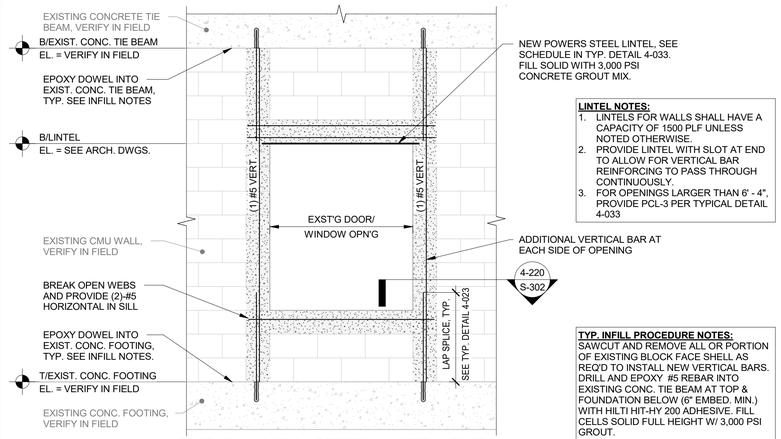
4-033



PROCEDURE NOTES:
1. DIG EXTERIOR OF BUILDING TO EXPOSE EXISTING FOUNDATION.
2. LAYOUT THE ADDED BARS LOCATION AS SHOWN ON PLAN.
3. BREAK THE EXTERIOR FACE OF CMU WALL TO INSTALL DOWEL (REBAR). DRILL & EPOXY DOWEL (REBAR) INTO EXISTING FOOTING (6" MIN. EMBED.) WITH HILT HIT-HY 200 EPOXY OR APPROVED EQUIVALENT. BEND BAR INTO THE CELL SPACING. (REPEAT PROCEDURE ON THE TOP UNDER TIE BEAM.
4. ON TOP OF THE WALL BREAK EXTERIOR FACE OF CMU WALL AS NEEDED TO INSTALL THE NEW ADDED VERTICAL REBAR.
5. FORM ALL OPENINGS DONE TO THE EXTERIOR WALL UP TO THE TOP ONE UNDER THE TIE BEAM. FILL CELLS WITH 3,000 PSI CONCRETE.
6. AFTER CONCRETE REACH 75% OF ITS DESIGN STRENGTH, DRY PACK THE TOP CELL WITH NON-SHRINK, NON-METALIC GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI.

REINFORCING EXISTING MASONRY WALL DETAIL

4-201



LINTEL NOTES:
1. LINTELS FOR WALLS SHALL HAVE A CAPACITY OF 1500 PLF UNLESS NOTED OTHERWISE.
2. PROVIDE LINTEL WITH SLOT AT END TO ALLOW FOR VERTICAL BAR REINFORCING TO PASS THROUGH CONTINUOUSLY.
3. FOR OPENINGS LARGER THAN 6' - 4", PROVIDE PCL-3 PER TYPICAL DETAIL 4-033

TYP. INFILL PROCEDURE NOTES:
SAWCUT AND REMOVE ALL OR PORTION OF EXISTING BLOCK FACE SHELL AS REQ'D TO INSTALL NEW VERTICAL BARS. DRILL AND EPOXY #5 REBAR INTO EXISTING CONC. TIE BEAM AT TOP & FOUNDATION BELOW (6" EMBED. MIN.) WITH HILTI HIT-HY 200 ADHESIVE. FILL CELLS SOLID FULL HEIGHT W/ 3,000 PSI GROUT.

TYPICAL NEW STEEL LINTEL OVER ALL EXISTING OPENINGS IN EXISTING CMU WALLS

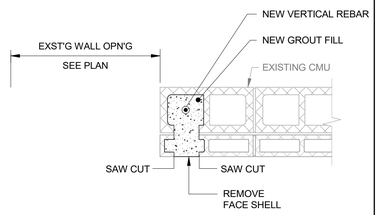
4-214

EXISTING MASONRY WALL CRACK REPAIR NOTES:

1. ALL CRACK REPAIRS SHALL BE PERFORMED AFTER FOUNDATION STABILIZATION IS COMPLETE.
2. REPLACE ALL DAMAGED AND/OR CRACKED BLOCKS/BRICKS WITH NEW BLOCKS/BRICKS AND RE-POINT AS REQUIRED.
3. RE-POINT ALL DAMAGED/DETERIORATED MORTAR.
4. INSTALL HELIFIX CRACK STITCHING AT ALL CRACK LOCATIONS
   - A. HELIBAR STITCHING ANCHORS SHALL BE STAINLESS STEEL 304 AND INSTALLED IN HELIBOND GROUT IN EVERY OTHER MORTAR JOINT VERTICALLY THROUGHOUT THE EXTENTS OF THE CRACK PER MANUF. SPECIFICATIONS.
   - B. HELIBARS SHALL EXTEND 20" MIN BEYOND THE CRACK ACH SIDE, TYPICAL.
   - C. RE-POINT THE MORTAR BED WITH TYPE "S" MORTAR.
   - D. WHERE A CRACK IS LESS THAN 20" FROM THE END OF A WALL, THE HELIBAR SHOULD BE BENT AROUND THE CORNER IN BONDED INTO THE RETURN WALL MIN 6".
   - E. WHERE A CRACK IS LESS THAN 20" FROM A TERMINATION POINT (CHASES, REVEALS, PILASTERS, ETC.) THE HELIBAR SHOULD BE BENT 90 DEGREES AND INSERTED INTO A 1-1/4" DIAMTER ENTRY HOLE TO A DEPTH OF 4".
   - F. IF TWO OR MORE CRACKS ARE CLOSE TOGETHER, THESE CAN BE STITCHED USING ONE CONTINUOUS LENGTH OF ROD.

EXISTING MASONRY WALL CRACK REPAIR NOTES

4-900



TYP. REINF. AT EXISTING CMU WALL OPEN'G JAMB

4-220

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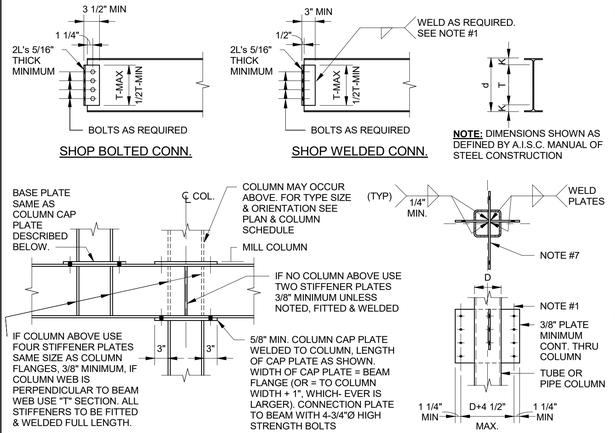
TYPICAL DETAILS

S-302



NOTES:

1. ALL CONNECTIONS SHALL BE DESIGNED FOR MAXIMUM UNIFORM LOAD CARRYING CAPACITY UNLESS GREATER REACTION IS SHOWN ON PLAN. IN ANY CASE, CONNECTIONS. SHALL NOT BE LESS THAN STANDARD A.I.S.C. CONNECTION FOR THE PARTICULAR BEAM. DEPTH.
2. DETAILER SHALL SUBMIT FOR APPROVAL, STANDARD CONNECTION DETAILS CONFORMING TO ABOVE DETAILS W/ERECTION DRAWING. SUBMISSION.
3. MILL COLUMNS. TO BEAR OR PROVIDE FULL PENETRATION WELD AT BASE PLATES & CAP PLATES AT COLUMNS CARRYING BEAMS.
4. AT CONNECTION OF HUNG BEAM. TO CANTILEVERED BEAMS. PROVIDE STANDARD (2) ANGLE CONNECTION AS PER NOTE #1 FOR LOAD CAPACITY OF HUNG BEAM.
5. WHERE BEAMS FRAME TO FITTED STIFFENERS AT COLUMNS PROVIDE SHEAR PLATE CONNECTION W/ STRENGTH CAPACITY AS PER NOTE #1.
6. ALL BOLTS TO BE MINIMUM 3/4"Ø TWIST-OFF TENSION CONTROLLED BOLTS CONFORMING TO ASTM F1852 OR F2280. UNLESS NOTED OTHERWISE ALL WELDING ELECTRODES TO BE E70 XX.
7. INSTALL THRU PLATE FOR BEAMS WITH LARGER REACTION FIRST AND WELD PLATES W/ LOWER REACTIONS TO IT.

NOTE: DIMENSIONS SHOWN AS DEFINED BY A.I.S.C. MANUAL OF STEEL CONSTRUCTION

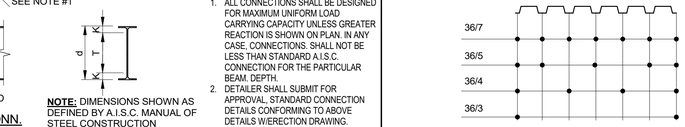
SHOP BOLTED CONN.

SHOP WELDED CONN.

BEAM OVER COLUMN AND COLUMN OVER BEAM

FRAMED CONNECTION AT TUBE/PIPE COLUMN

5-001



EXAMPLE

36/7-4

- NO. OF SIDE LAP FASTENERS
- NO. OF SUPPORT FASTENERS
- WIDTH OF METAL DECK

NOTES:

1. USE #12 SCREWS AT SUPPORTS AND #12 SCREWS AT SIDE LAPS.
2. USE 36/7-4 UNLESS NOTED OTHERWISE IN PLANS.
3. USE #12 SCREWS @ 6" O.C. ALONG DECK CLOSURE UNLESS NOTED OTHERWISE IN PLANS.

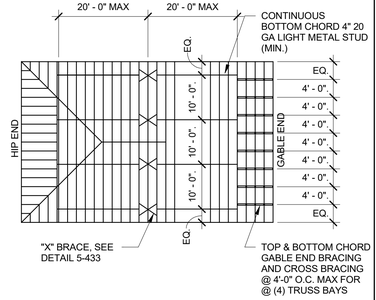
TYPICAL 1-1/2" ROOF DECK FASTENER LAYOUT

5-306

EXTERIOR METAL STUD WALL DESIGN NOTES:

1. CALCULATE PROPERTIES AND LOAD CAPACITIES OF MEMBERS PER THE AISI SPECIFICATIONS FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
2. DESIGN EXTERIOR CURTAIN WALL TO SUSTAIN WIND LOADS INDICATED IN THE SCHEDULE OF LOADS DRAWING.
3. LIMIT LATERAL DEFLECTIONS UNDER DESIGN LOADS TO L/800 OR 3/8" WHICH EVER IS SMALLER FOR THOSE AREAS WITH BRICK CLADDING AND L/360 FOR ALL OTHER AREAS.
4. PROVIDE SLIDE CLIPS OR DEFLECTION TRACKS ON THE UNDERSIDE OF THE ELEVATED LEVELS TO ALLOW FOR A MAXIMUM LIVE LOAD VERTICAL DEFLECTION OF APPROXIMATELY L/240 AT MID SPAN BETWEEN COLUMNS.
5. DIAGONAL KICKERS ARE NOT ALLOWED, UNLESS APPROVED IN WRITING BY ARCHITECT AND ENGINEER OF RECORD.
6. DESIGN ALL CONNECTIONS TO SUSTAIN THE MAXIMUM DESIGN LOADS.
7. ALL DETAILS DRAWINGS SHOWN ARE MINIMUMS REQUIRED FOR PARTICULAR CONDITIONS. CONTRACTOR IS RESPONSIBLE TO SUBMIT SIGN AND SEALED DRAWINGS AND CALCULATIONS FOR A WALL SYSTEM THAT COMPLIES WITH THESE NOTES, THE SPECIFICATIONS AND THE CONTRACT DOCUMENTS INCLUDING BUT NOT LIMITED TO STRUCTURAL AND ARCHITECTURAL DRAWINGS.

5-400



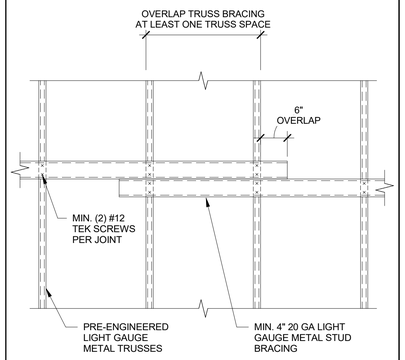
NOTES:

1. SEE TRUSS MANUFACTURER'S TRUSS ENGINEERING CUT SHEETS FORM ANY ADDITIONAL PERMANENT BRACING/S REQUIRED.
2. "T" BRACING MAY BE USED IN PLACE OF PERMANENT BRACING PROVIDED IT EXTENDS OVER AT LEAST 90% OF THE WEB.

REQUIRED MINIMUM PERMANENT TRUSS BRACING PLAN

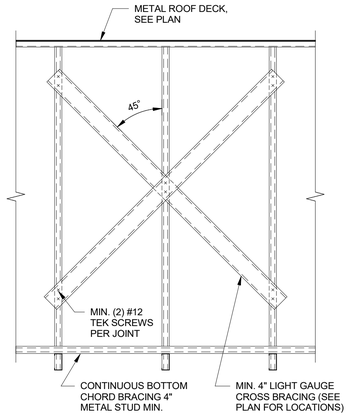
5-431

NOTE:
IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWINGS ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.



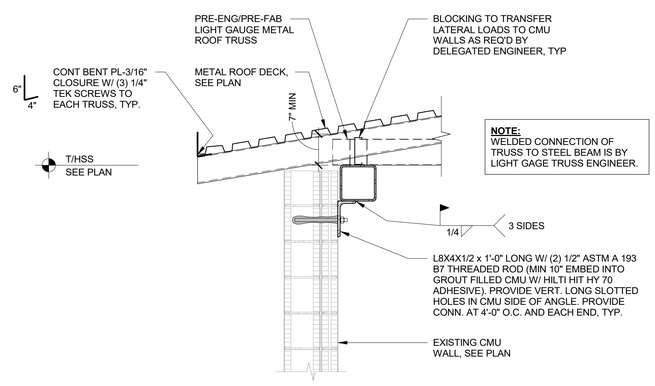
TRUSS BRACING OVERLAP DETAIL

5-432



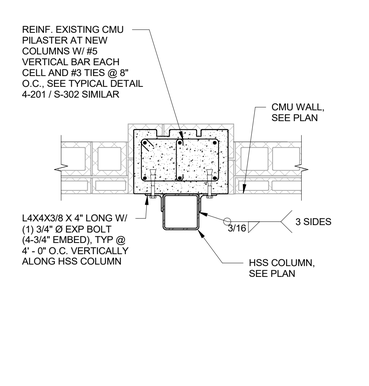
TYPICAL CROSS BRACE DETAIL

5-433



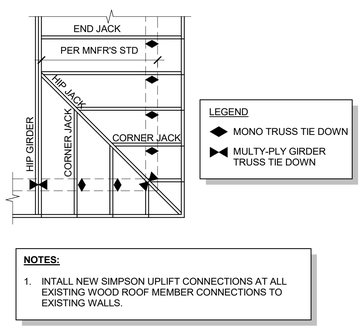
TYPICAL DETAIL AT NEW LIGHT GAUGE TRUSS BEARING AT AUDITORIUM

5-434



NEW HSS COLUMN CONNECTION TO EXISTING CMU WALL

5-501

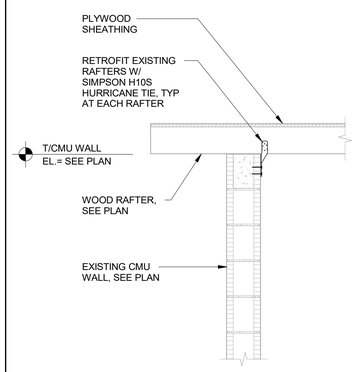


NOTES:

1. INTALL NEW SIMPSON UPLIFT CONNECTIONS AT ALL EXISTING WOOD ROOF MEMBER CONNECTIONS TO EXISTING WALLS.

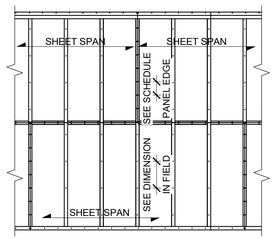
RETROFIT UPLIFT CONNECTION LOCATIONS AT EXISTING WOOD ROOF FRAMING

6-006



RETROFIT UPLIFT CONNECTION DETAIL AT EXISTING WOOD ROOF FRAMING

6-007



NOTES:

1. SEE STRUCTURAL NOTES FOR DECKING SPECIFICATIONS.
2. END JOINTS OF FLOOR DECKING SHALL BE STAGGERED AND OCCUR OVER FRAMING MEMBER.
3. PROVIDE INTERMEDIATE BLOCKING AT JOINT LOCATIONS WHERE NO FRAMING OCCURS.
4. CONTINUOUSLY GLUE DECKING TO FLOOR JOIST/TRUSS.
5. DECKING & NAILING SCHEDULE:
   23/32" 10d COMMON NAILS @ 4" O.C. @ PANEL EDGES.
   23/32" 10d COMMON NAILS @ 12" O.C. IN FIELD.

FLOOR DECKING NAILING SCHEDULE

6-011

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STEVIE W. LOCKHART, P.E.
FL Lic. No. 86402
PROJ NO. 3104-169-11
CA: 8426

TO THE BEST OF OUR KNOWLEDGE, INFORMATION AND BELIEF, THESE STRUCTURAL PLANS CONFORM TO AND SATISFY, THE FLORIDA BUILDING CODE, EIGHTH EDITION 2023, ACI 318-19 AND LOCAL CODES AS APPLICABLE

PRELIMINARY NOT FOR CONSTRUCTION

THELMA BOLTIN CENTER
CITY OF GAINESVILLE
516 NE 2nd Ave, Gainesville, FL 32601

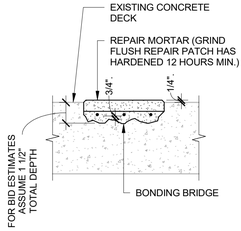
Project number
2018

90% Review Set 04/11/2025

REV	DESCRIPTION	DATE

TYPICAL DETAILS

S-303



SURFACE PREPARATION

1. DETERMINE THE APPROXIMATE EDGE OF THE HOLLOW SOUNDING CONCRETE.
2. SAW CUT 1/4" DEEP THE PERIMETER OF AREA DAMAGED. TAKE SPECIAL CARE NOT TO CUT ANY REINFORCEMENT.
3. USING HAND TOOLS CHIP ALL UNSOUND CONCRETE WITHIN THE DAMAGE AREA. IF POST- TENSIONED CABLES ARE EXPOSED, STOP CHIPPING CONCRETE AND CONTACT THE ENGINEER. CHIP THE SURFACE TO OBTAIN A ROUGH SURFACE PROFILE WITH A MINIMUM DEPTH OF 1/2". IF EXPOSED DEPTH EXCEEDS 1-1/2" IN DEPTH, CONTACT THE ENGINEER.
4. WHERE MORE THAN HALF OF THE DIAMETER OF THE REINFORCEMENT BAR IS EXPOSED, USE HAND TOOLS TO REMOVE A MINIMUM OF 3/4" BEHIND THE REINFORCEMENT.
5. REINFORCING STEEL MUST BE MECHANICAL CLEANED TO REMOVE RUST, GREASE, OIL, & OTHER BOND-INHIBITING MATTER BY SAND-BLASTING.
6. IF REINFORCING BAR CROSS-SECTION AREA HAS BEEN REDUCED BY MORE THAN 20% CONTACT THE ENGINEER FOR INSTRUCTION.

REPAIR PROCEDURE:

SIKA PRODUCTS:

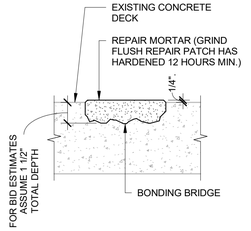
1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY SIKA ARMATEC 110 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. CURE TO TACK- FREE 2-3 HOURS.
3. APPLY A SECOND COAT AT 10 MILS. COVERING ALL EXPOSED STEEL AND AREA TO BE PATCH. ALLOW TO DRY FOR 2-3 HOURS.
4. WORK A SCRUB COAT OF SIKATOP 122 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
5. APPLY SIKATOP 122 MORTAR. FOR APPLICATIONS GREATER THAN 1" IN DEEP, ADD A 3/8" COARSE AGGREGATE. PATCH MATERIAL SHOULD BE FINISHED APPROXIMATELY 1/4" HIGHER THAN SURROUNDING CONCRETE.
6. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.
7. AFTER THE REPAIR MORTAR HAS REACHED ITS DESIGN STRENGTH, APPROXIMATELY 72 HOURS, GRIND SURFACE SMOOTH TO MATCH SURROUNDING CONCRETE.

STO PRODUCTS:

1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY STO CR246 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. ALLOW TO DRY FOR 30 - 45 MINUTES.
3. APPLY A SECOND COAT AT 10 MILS. COVERING ALL EXPOSED STEEL AND AREA TO BE PATCH. ALLOW TO DRY FOR 2-3 HOURS.
4. WORK A SCRUB COAT OF STO CR730 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
5. APPLY STO FLOWABLE MORTAR CR730. FOR APPLICATIONS GREATER THAN 1" IN. DEEP, ADD A 3/8" COARSE AGGREGATE. PATCH MATERIAL SHOULD BE FINISHED APPROXIMATELY 1/4" HIGHER THAN SURROUNDING CONCRETE.
6. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.
7. AFTER THE REPAIR MORTAR HAS REACHED ITS DESIGN STRENGTH, APPROXIMATELY 72 HOURS, GRIND SURFACE SMOOTH TO MATCH SURROUNDING CONCRETE.

CONCRETE DECK SPALL W/ EXPOSED STEEL

9-001



SURFACE PREPARATION

1. DETERMINE THE APPROXIMATE EDGE OF THE HOLLOW SOUNDING CONCRETE.
2. SAW CUT 1/4" DEEP THE PERIMETER OF AREA DAMAGED. TAKE SPECIAL CARE NOT TO CUT ANY REINFORCEMENT.
3. USING HAND TOOLS CHIP ALL UNSOUND CONCRETE WITHIN THE DAMAGE AREA. IF POST- TENSIONED CABLES ARE EXPOSED, STOP CHIPPING CONCRETE AND CONTACT THE ENGINEER. CHIP THE SURFACE TO OBTAIN A ROUGH SURFACE PROFILE WITH A MINIMUM DEPTH OF 1/2". IF EXPOSED DEPTH EXCEEDS 1-1/2" IN DEPTH, CONTACT THE ENGINEER.

REPAIR PROCEDURE:

SIKA PRODUCTS:

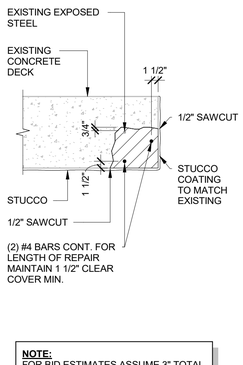
1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY SIKA ARMATEC 110 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. CURE TO TACK- FREE 2-3 HOURS.
3. WORK A SCRUB COAT OF SIKATOP 122 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
4. APPLY SIKATOP 122 MORTAR. FOR APPLICATIONS GREATER THAN 1" IN DEEP, ADD A 3/8" COARSE AGGREGATE. PATCH MATERIAL SHOULD BE FINISHED APPROXIMATELY 1/4" HIGHER THAN SURROUNDING CONCRETE.
5. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.
6. AFTER THE REPAIR MORTAR HAS REACHED ITS DESIGN STRENGTH, APPROXIMATELY 72 HOURS, GRIND SURFACE SMOOTH TO MATCH SURROUNDING CONCRETE.

STO PRODUCTS:

1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY STO CR246 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. ALLOW TO DRY FOR 30 - 45 MINUTES.
3. WORK A SCRUB COAT OF STO CR730 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
4. APPLY STO FLOWABLE MORTAR CR730. FOR APPLICATIONS GREATER THAN 1" IN. DEEP, ADD A 3/8" COARSE AGGREGATE. PATCH MATERIAL SHOULD BE FINISHED APPROXIMATELY 1/4" HIGHER THAN SURROUNDING CONCRETE.
5. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.
6. AFTER THE REPAIR MORTAR HAS REACHED ITS DESIGN STRENGTH, APPROXIMATELY 72 HOURS, GRIND SURFACE SMOOTH TO MATCH SURROUNDING CONCRETE.

CONCRETE DECK SPALL W/O EXPOSED STEEL

9-002



NOTE:
FOR BID ESTIMATES ASSUME 3" TOTAL DEPTH VERTICAL AND HORIZONTAL

SURFACE PREPARATION

1. DETERMINE THE APPROXIMATE EDGE OF THE HOLLOW SOUNDING CONCRETE.
2. SAW CUT 1/4" DEEP THE PERIMETER OF AREA DAMAGED. TAKE SPECIAL CARE NOT TO CUT ANY REINFORCEMENT.
3. USING HAND TOOLS CHIP ALL UNSOUND CONCRETE WITHIN THE DAMAGE AREA. IF POST- TENSIONED CABLES ARE EXPOSED, STOP CHIPPING CONCRETE AND CONTACT THE ENGINEER. CHIP THE SURFACE TO OBTAIN A ROUGH SURFACE PROFILE WITH A MINIMUM DEPTH OF 1/2". IF EXPOSED DEPTH EXCEEDS 1-1/2" IN DEPTH, CONTACT THE ENGINEER.
4. WHERE MORE THAN HALF OF THE DIAMETER OF THE REINFORCEMENT BAR IS EXPOSED, USE HAND TOOLS TO REMOVE A MINIMUM OF 3/4" BEHIND THE REINFORCEMENT.
5. REINFORCING STEEL MUST BE MECHANICAL CLEANED TO REMOVE RUST, GREASE, OIL, & OTHER BOND-INHIBITING MATTER BY SAND-BLASTING.
6. IF REINFORCING BAR CROSS-SECTION AREA HAS BEEN REDUCED BY MORE THAN 20% CONTACT THE ENGINEER FOR INSTRUCTION.
7. INSTALL FORMS IF REQUIRED THAT ARE LINED OR COATED WITH RELEASE AGENT FOR EASY REMOVAL. SPECIAL CARE MUST BETAKEN WHEN APPLYING FORM RELEASE AGENT TO THE FORMS SO NONE OF THE STEEL OR EXISTING CONCRETE GETS COATED WITH IT.

REPAIR PROCEDURE:

SIKA PRODUCTS:

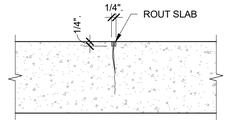
1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY SIKA ARMATEC 110 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. CURE TO TACK- FREE 2-3 HOURS.
3. APPLY A SECOND COAT AT 10 MILS. COVERING ALL EXPOSED STEEL AND AREA TO BE PATCH. ALLOW TO DRY FOR 2-3 HOURS.
4. WORK A SCRUB COAT OF SIKATOP 122 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
5. WHILE SCRUB COAT IS STILL WET, APPLY SIKATOP 123 MORTAR. FOR APPLICATIONS GREATER THAN 1-1/2" IN. DEEP, APPLY SIKATOP 123 IN LIFTS. SCORE THE TOP SURFACE OF EACH LIFT TO PRODUCE A ROUGHENED SURFACE FOR THE NEXT LIFT. ALLOW PROCEEDING LIFT TO REACH FINAL SET REPEAT FROM STEP 4.
6. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.

STO PRODUCTS:

1. PRE-WET THE SURFACE TO SATURATED SURFACE DRY.
2. APPLY STO CR246 WITH STIFF BRISTLE BRUSH OR SPRAY 10 MILS THICK COVERING ALL EXPOSED STEEL. ALLOW TO DRY FOR 30 - 45 MINUTES.
3. APPLY A SECOND COAT AT 10 MILS. COVERING ALL EXPOSED STEEL AND AREA TO BE PATCH. ALLOW TO DRY FOR 2-3 HOURS.
4. WORK A SCRUB COAT OF STO CR702 MORTAR INTO THE SUBSTRATE TO ENSURE INTIMATE CONTACT AND ESTABLISH BOND.
5. WHILE SCRUB COAT IS STILL WET, APPLY STO OVERHEAD MORTAR CR702. FOR APPLICATIONS GREATER THAN 1-1/2" IN. DEEP, APPLY STO OVERHEAD MORTAR IN LIFTS. SCORE THE TOP SURFACE OF EACH LIFT TO PRODUCE A ROUGHENED SURFACE FOR THE LIFT. ALLOW PRECEDING LIFT TO REACH FINAL SET REPEAT FROM STEP 4.
6. USE A WATER BASED CURING COMPOUND THAT MEETS ASTM C-309 OR USE CONTINUOUS LIGHT WATER FOGGING FOR 48 HOURS.

CONCRETE EDGE REPAIR OF CRACKS & SPALLS

9-003



SURFACE PREPARATION:

1. VEE NOTCH THE SURFACE OF THE CRACK WITH A MECHANICAL ROUTER OR HAND CHIPPING TOOL TO A MAXIMUM WIDTH OF A 1/4". REMOVE LOOSE DEBRIS.
2. REMOVE ALL DUST, LAITANCE, GREASE, CURING COMPOUND, WAXES, IMPREGNATION'S, FOREIGN PARTICLES, EFFLORESCENCE AND OTHER BOND INHIBITING MATERIALS FROM THE SURFACE BY MECHANICAL MEANS, I.E. - SANDBLASTING, HIGH PRESSURE WATER BLASTING, ETC.., AS APPROVED BY ENGINEER.
3. SEAL UNDERSIDE OF SLAB IF CRACKS REFLECT THROUGH WITH AN EPOXY GEL.

REPAIR PROCEDURE:

SIKA PRODUCTS:

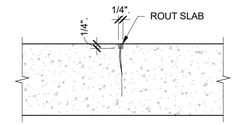
1. POUR NEAT SIKADUR 35 HI-MOD LV EPOXY RESIN ADHESIVE INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
2. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

STO PRODUCTS:

1. POUR NEAT STO EPOXY BINDER CR633 INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
2. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

TYPICAL SECTION - CRACK TYPE I (20 MILLS TO 1/4")

9-004



SURFACE PREPARATION:

1. VEE NOTCH THE SURFACE OF THE CRACK WITH A MECHANICAL ROUTER OR HAND CHIPPING TOOL TO A MAXIMUM WIDTH OF A 1/2". REMOVE LOOSE DEBRIS.
2. REMOVE ALL DUST, LAITANCE, GREASE, CURING COMPOUND, WAXES, IMPREGNATION'S, FOREIGN PARTICLES, EFFLORESCENCE AND OTHER BOND INHIBITING MATERIALS FROM THE SURFACE BY MECHANICAL MEANS, I.E. - SANDBLASTING, HIGH PRESSURE WATER BLASTING, ETC., AS APPROVED BY ENGINEER.
3. SEAL UNDERSIDE OF SLAB IF CRACKS REFLECT THROUGH.

REPAIR PROCEDURE:

SIKA PRODUCTS:

1. PRIOR TO PRODUCT APPLICATION SUBSTRATE SHOULD BE SATURATED SURFACE DRY (SSD) WITH NO STANDING WATER.
2. POUR NEAT SIKA TOP 111 INTO VEE NOTCHED CRACK. CONTINUE PLACEMENT UNTIL CRACK IS COMPLETELY FILLED.
3. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

STO PRODUCTS:

1. FILL CRACK WITH OVEN-DRIED SAND PRIOR TO THE APPLICATION OF THE STO EPOXY BINDER.
2. POUR NEAT STO EPOXY BINDER CR633 INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
3. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

TYPICAL SECTION - CRACK TYPE II (1/4" OR GREATER)

9-005

NOTE:
IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWINGS ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.

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PRELIMINARY NOT FOR CONSTRUCTION

THELMA BOLTIN CENTER

CITY OF GAINESVILLE
516 NE 2nd Ave, Gainesville, FL 32601

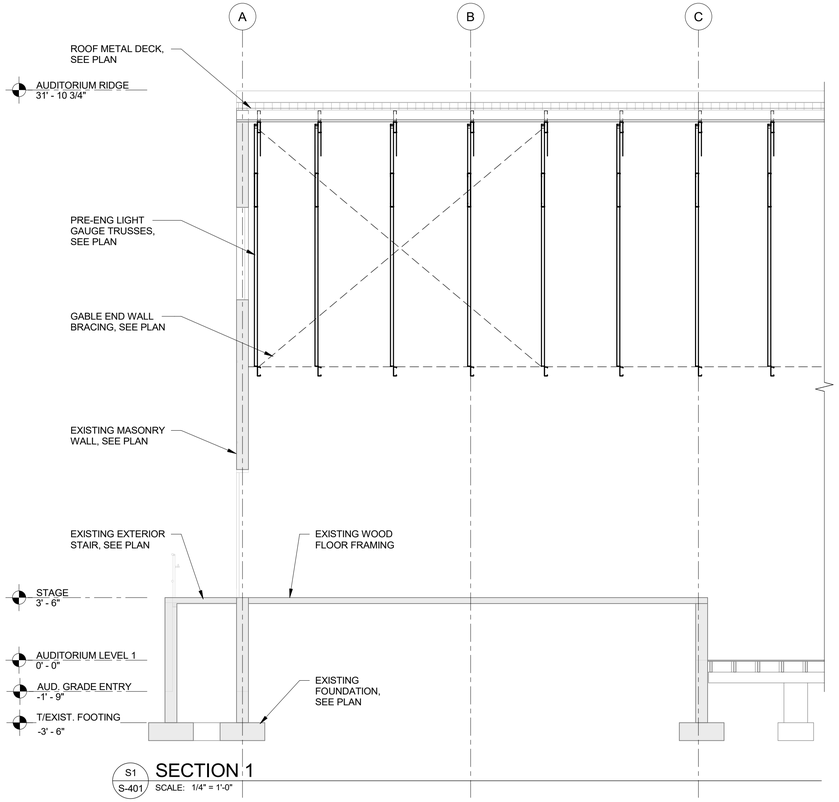
Project number
2018

90% Review Set 04/11/2025

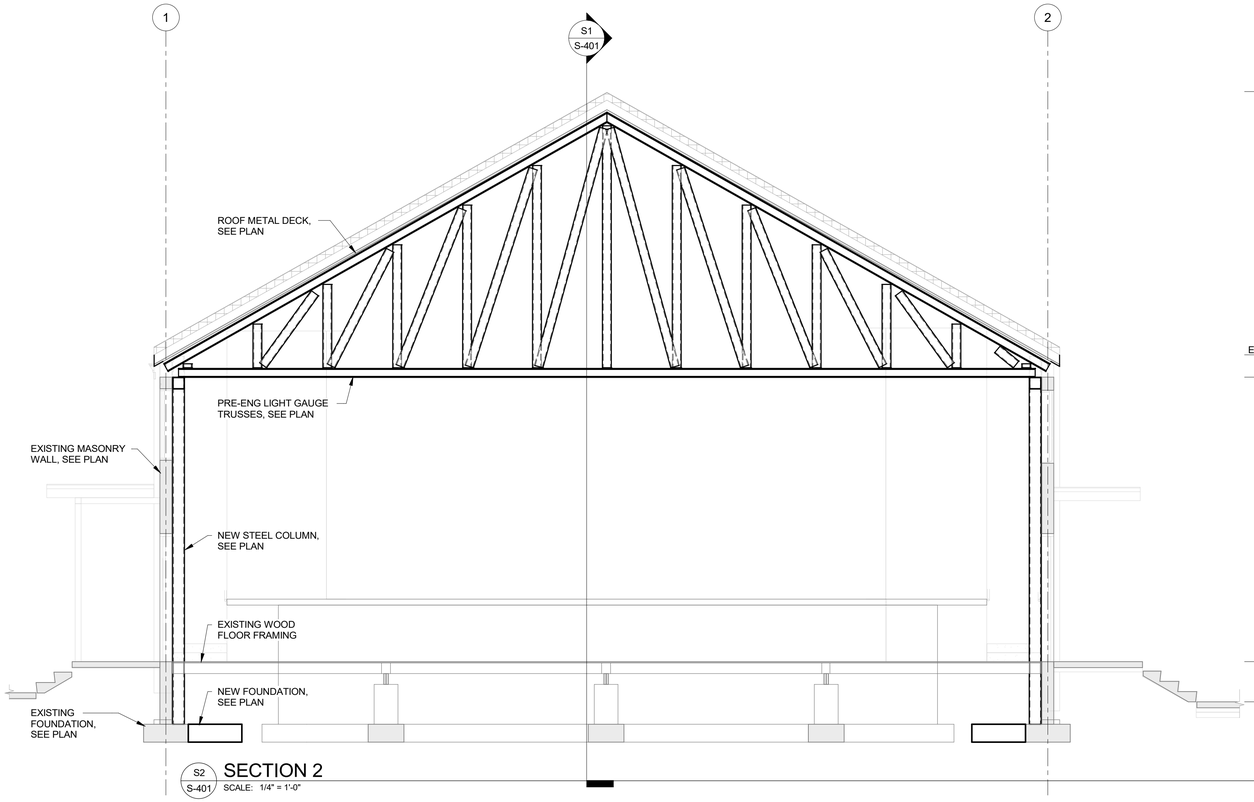
REV	DESCRIPTION	DATE

TYPICAL DETAILS

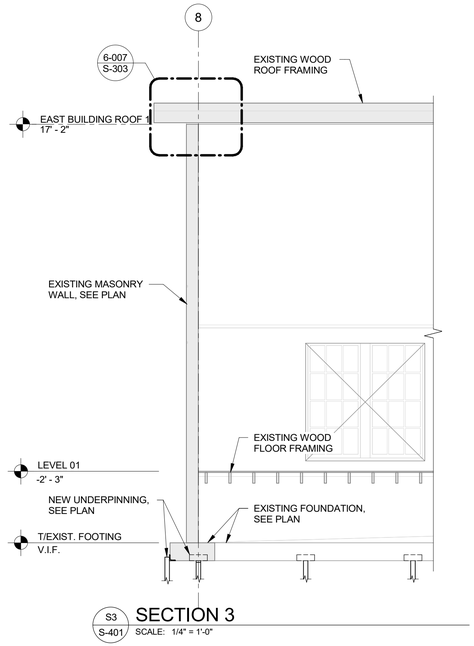
S-304



S1 / S-401 SECTION 1
SCALE: 1/4" = 1'-0"



S2 / S-401 SECTION 2
SCALE: 1/4" = 1'-0"



S3 / S-401 SECTION 3
SCALE: 1/4" = 1'-0"

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SECTIONS

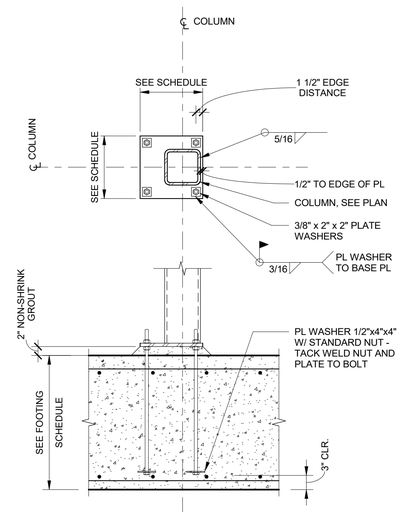
S-401

T/HSS																	T/HSS
15' - 11"	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	HSS8X8X5/8	15' - 11"
FOUNDATION																	FOUNDATION
-3' - 6"	S1	S2	S3	S4	S5	S6	S7	S8	S9	S10	S11	S12	S13	S14	S15	S16	-3' - 6"
Column Locations	A(-0' - 10")-1(-0' - 8 1/2")	A(-0' - 10")-2(0' - 8 1/2")	B-1(-0' - 8 1/2")	B-2(0' - 8 1/2")	C-1(-0' - 8 1/2")	C-2(0' - 8 1/2")	D-1(-0' - 8 1/2")	D-2(0' - 8 1/2")	E-1(-0' - 8 1/2")	E-2(0' - 8 1/2")	F-1(-0' - 8 1/2")	F-2(0' - 8 1/2")	G-1(-0' - 8 1/2")	G-2(0' - 8 1/2")	H-1(-0' - 8 1/2")	H-2(0' - 8 1/2")	
BASE PLATE	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	BP-1	

STEEL COLUMN SCHEDULE

SCALE: 1/8" = 1'-0"

BASE PLATE SCHEDULE			
BASE PLATE	**SIZE**	**ANCHOR BOLTS**	**REMARKS**
BP-1	3/4"x14"x14"	(4) 3/4"ø F1554 GR. 36	



BP-1 / S-601 BASE PLATE - BP-1

SCALE: 3/4" = 1'-0"

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SCHEDULES

S-601